STORMWATER MANAGEMENT PLAN

Tax Map 31 Lot 48 Tax Map 33 Lot 18 Kearsarge Mountain Road Warner, NH

STORMWATER MANAGEMENT REPORT

PROPOSED RESIDENTIAL DEVELOPMENT KEARSARGE MOUNTAIN ROAD WARNER, NH

June 28, 2023 February 21, 2024 July 24, 2024 August 4, 2024

PREPARED FOR:

Sydney Elizabeth Boyer Kearsarge Mountain Road Warner NH 03278 JON BOKEH NO. 11660 P. JONNAL ENGINEER

PREPARED BY:

Jon Rokeh, P.E. Rokeh Consulting, LLC 89 King Road, Chichester, NH Phone: 603-387-8688

INDEX

INTRODUCTION

EXISTING CONDITIONS
PROPOSED CONDITIONS
METHODOLOGY
SUMMARY
USGS
AERIAL & SITE PHOTOS
NRCS WEBB SOIL SURVEY
SSSNNE PUBLICATION #5
NORTHEAST REGIONAL CLIMATE CENTER
BMP WORK SHEETS
OUTLET PROTECTION
PREDEVELOPMENT DRAINAGE CALCULATIONS
POST DEVELOPMENT DRAINAGE CALCULATIONS
OPERATION AND MAINTENANCE PLAN

INTRODUCTION

The proposed project is located on Map 33 Lot 18 off of Kearsarge Mountain Road in Warner NH. The lot is a 10+ acre parcel moderately sloping and wooded parcel of land.

The project consists of a 24 foot roadway 700 feet long and ending in a culdesac turn around. The roadway will provide access to 4 new single family lots. The lots will be serviced municipal sewer and water.. The total area disturbed during construction of the road is approximately 68,000 SF.

The intent of this drainage report is to provide calculations for the proposed drainage improvements and evaluate pre and post development conditions in accordance with the Town of Warner Subdivision Regulations and NHDES requirements.

EXISTING CONDITIONS

The property is currently a undeveloped moderately sloping wood lot with some small areas of wetland along the frontage and to the west of the proposed developed area. On average the slopes vary from 3 to 25%. The entire property was mapped as 55C Hermon sandy loam series with 8% to 25% slopes as derived from NRCS Web Soil Survey.

The property has 2 distinct flow patterns as shown on the predevelopment drainage area plan. Design node 1 is in the southerly corner of the lot where the existing drainage from the wetland noted above flows in the corner of the lot and under Kearsarge Mt Road. The other Design node, node 2 is located in the easterly corner of the property with the existing drainage flowing from North to south into the exiting wetlands and then to the east as seen in the existing drainage area plan.

PROPOSED CONDITIONS

The proposed will consist of a 700+ foot roadway providing access and frontage to 4 new single family homes. The homes will be serviced by both municipal water and sewer. All of the stormwater generated from the site will be directed primarily by open drainage to two micro pool extended detention ponds and one infiltration trench located at CB2. The ponds will be preceded by forebays for sediment removal prior stormwater entering the ponds. The ponds have been design to treat the water quality volume before detaining and releasing the stormwater to the exiting site. Micro-pool ponds use extended detention and a wet micro pool for treatment. The ponds are located adjacent to wetlands making them ideal for this treatment condition. The one infiltration trench is located at CB 2 it consists of a 4'x6' stone trench with a perferorated pipe. A infiltration rate of 3 inches per hour is used. This rate is 50% of the slowest rate in the C horizon or 50% of 6 inches as depicted in SSSNNE Special Publication No. 5.

METHODOLOGY

The drainage analysis was completed using HydroCad Version 10.00-22, a stormwater modeling program utilizing TR-20 and TR-55 methodology. This program performs both the hydrologic

computations for determination of runoff flows, and the hydraulic calculations for pipe, ditch, and pond design. Calculations were performed for the 2, 10, 25 and 50-year frequency storms in accordance with Town and NHDES regulations.

The following design parameters were used:

Type III
2.76 inches
4.02 inches
4.98 inches
5.87 inches

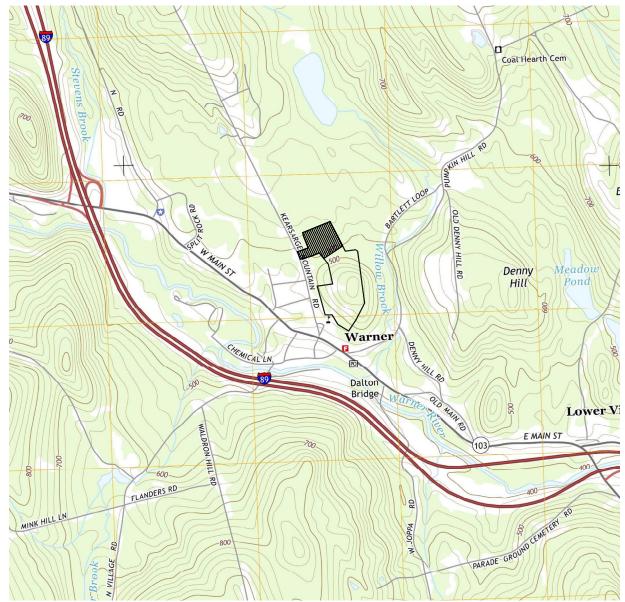
FINDINGS:

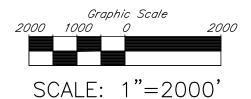
Based on the results of this analysis we expect that the project will not adversely affect any downstream properties or receiving waters.

	CFS	CFS	CFS	CFS
	2 YEAR	10 YEAR	25 YEAR	50 YEAR
POA1				
Pre-Development	0.00	0.00	0.03	0.16
Post Development	0.00	0.00	0.02	0.14
POA2				
Pre-Development	0.00	0.00	0.04	0.22
Post Development	0.00	0.00	0.04	0.19

USGS







Owner / Applicant:
Sydney Elizabeth Boyer
Kearsarge Mountain Road
Warner, NH 03278

JUNE 28, 2023

USGS TOPOGRAPHIC MAP
PROPOSED LAND SUBDIVISION
TAX MAP 31 LOT 48
TAX MAP 33 LOT 18
Kearsarge Mountain Road
Warner, New Hampshire

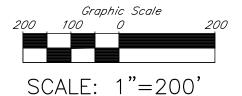
PREPARED BY:

Rokeh Consulting, LLC 89 KING ROAD, CHICHESTER, NH PH: 603-387-8688

AERIAL & SITE PHOTOS







Owner / Applicant: Sydney Elizabeth Boyer Kearsarge Mountain Road Warner, NH 03278

JUNE 28, 2023

AERIAL PHOTO
PROPOSED LAND SUBDIVISION
TAX MAP 31 LOT 48
TAX MAP 33 LOT 18
Kearsarge Mountain Road
Warner, New Hampshire

PREPARED BY:

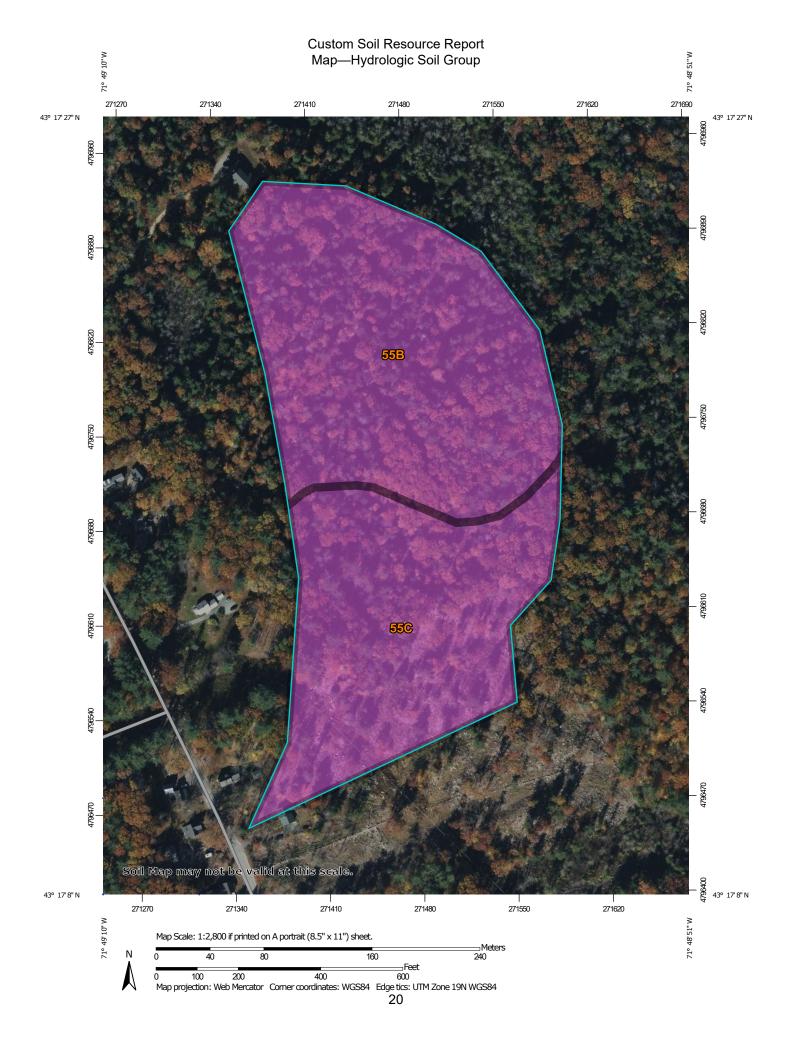
Rokeh Consulting, LLC 89 KING ROAD, CHICHESTER, NH PH: 603-387-8688





NRCS WEBB SOIL SURVEY





43° 17'19" N

43° 17'8" N

6/24/2023 Page 1 of 4

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Enlargement of maps beyond the scale of mapping can cause Warning: Soil Map may not be valid at this scale.

contrasting soils that could have been shown at a more detailed misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of scale.

Please rely on the bar scale on each map sheet for map measurements. Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

distance and area. A projection that preserves area, such as the Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Merrimack and Belknap Counties, New Hampshire

Survey Area Data: Version 28, Sep 6, 2022

Soil map units are labeled (as space allows) for map scales

1:50,000 or larger.

Not rated or not available

B/D

S

Soil Rating Points

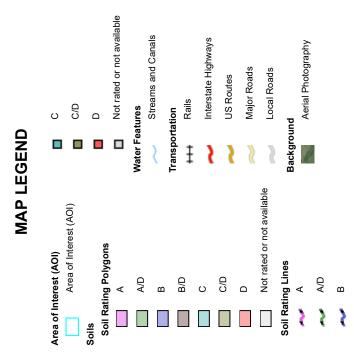
⋖

ΑD

B/D

Date(s) aerial images were photographed: Oct 6, 2022—Oct 22,

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
55B	Hermon sandy loam, 0 to 8 percent slopes, very stony	A	3.2	10.6%
55C	Hermon sandy loam, 8 to 15 percent slopes, very stony	A	23.2	76.2%
380D	Tunbridge-Lyman- Becket complex, 15 to 25 percent slopes, very stony	С	0.3	1.1%
394A	Chocorua mucky peat, 0 to 1 percent slopes	A/D	3.5	11.4%
415B	Moosilauke fine sandy loam, 3 to 8 percent slopes, very stony	A/D	0.2	0.7%
Totals for Area of Inter	rest	•	30.5	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

NORTHEAST REGIONAL CLIMATE CENTER

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Metadata for PointSmoothingYesStateStateLocation43.281 degrees NorthLatitude71.817 degrees WestElevation130 feetDate/TimeSat Jun 24 2023 13:36:45 GMT-0400 (Eastern Daylight Time)

Extreme Precipitation Estimates

	5min	10min	15min 30min	30min	60min 120min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.26	0.40	0.49	9.0	0.81	1.02	1yr	0.70	96.0	1.17	1.47	1.85	2.33	2.56	1yr	2.06	2.46	2.90	3.60	4.13	1yr
2yr	0.31	0.48	09.0	62.0	66.0	1.25	2yr	98.0	1.14	1.44	1.79	2.22	2.76	3.10	2yr	2.44	2.98	3.47	4.15	4.75	2yr
5yr	0.37	0.58	0.72	26.0	1.24	1.57	5yr	1.07	1.44	1.81	2.25	2.78	3.42	3.92	5yr	3.03	3.77	4.37	5.13	5.82	5yr
10yr	0.42	99.0	0.83	1.13	1.46	1.87	10yr	1.26	1.71	2.16	2.68	3.29	4.02	4.68	10yr	3.56	4.50	5.19	6.03	08.9	10yr
25yr	0.49	0.79	1.00	1.38	1.83	2.35	25yr	1.58	2.16	2.72	3.37	4.12	4.98	5.92	25yr	4.41	5.69	6.54	7.47	8.34	25yr
50yr	0.56	06.0	1.15	1.61	2.17	2.80	50yr	1.87	2.58	3.25	4.02	4.89	5.87	7.07	50yr	5.19	08.9	7.79	8.78	9.75	50yr
100yr	0.64	1.03	1.33	1.89	2.57	3.34	100yr	2.22	3.08	3.88	4.78	5.79	6.91	8.45	100yr	6.12	8.13	9.27	10.33	11.39	100yr
200yr	0.73	1.19	1.55	2.21	3.05	3.97	200yr	2.63	3.68	4.62	5.69	98.9	8.15	10.11	200yr	7.21	9.72	11.05	12.16	13.31	200yr
500yr	88.0	1.44	1.88	2.73	3.82	5.00	500yr	3.30	4.65	5.82	7.15	8:58	10.13	12.81	500yr	8.97	12.32	13.94	15.10	16.37	500yr

Lower Confidence Limits

					ľ																
	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.23	0.36	0.44	0.59	0.72	98.0	1yr	0.62	0.84	0.94	1.28	1.59	1.90	2.34	1yr	1.68	2.25	2.59	3.17	3.66	1yr
2yr	0.30	0.46	0.57	0.77	0.95	1.13	2yr	0.82	1.10	1.29	1.71	2.21	2.68	2.99	2yr	2.37	2.88	3.35	4.02	4.61	2yr
5yr	0.34	0.52	0.65	68.0	1.13	1.33	5yr	0.97	1.30	1.51	1.97	2.54	3.17	3.59	5yr	2.81	3.45	3.98	4.73	5.41	5yr
10yr	0.38	0.58	0.72	1.00	1.30	1.51	10yr	1.12	1.48	1.69	2.19	2.83	3.61	4.11	10yr	3.20	3.95	4.53	5.36	6.07	10yr
25yr	0.43	0.65	0.81	1.15	1.51	1.76	25yr	1.31	1.72	1.97	2.51	3.24	4.29	4.89	25yr	3.80	4.71	5.38	6.33	7.08	25yr
$50 \mathrm{yr}$	0.46	0.71	0.88	1.26	1.70	1.97	50yr	1.47	1.92	2.21	2.80	3.59	4.90	5.58	$50 \mathrm{yr}$	4.34	5.36	6.12	7.19	7.98	50yr
$100 \mathrm{yr}$	0.50	92.0	96.0	1.38	1.89	2.20	$100 \mathrm{yr}$	1.63	2.15	2.47	3.12	3.99	5.61	6.36	100yr	4.97	6.12	6.97	8.18	8.99	100yr
$200 \mathrm{yr}$	0.55	0.82	1.04	1.51	2.10	2.46	200yr	1.82	2.41	2.76	3.48	4.43	6.43	7.26	200yr	5.69	86.9	7.92	9.33	10.12	200yr
500 yr	0.62	0.92	1.18	1.71	2.43	2.83	$500 \mathrm{yr}$	2.10	2.77	3.20	4.04	5.10	7.72	8.63	500 yr	6.83	8.30	9.39	11.12	11.85	500yr

Upper Confidence Limits

	1
10day	7 V V
7day	2 N1
4day	216
2day	71 6 07 6 76 6
1day	フ レ レ
	1 4
48hr	020
24hr	1 02 0 25
12hr	JU 1
6hr	1 50
3hr	1 11
2hr	1 00
1hr	0L U
	1 4 4 4 4 4
120min	1 1 1
60min	UU U
in 15min 30min 60min	0 7 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
15min	22 U
10m	N 15
Smin 1	210 000
	1 4734

BMP WORK SHEETS



STORMWATER POND DESIGN CRITERIA

Env-Wq 1508.03

Type/Node Name: WET POND 1 2-20-24

Enter the type of stormwater pond (e.g., Wet Pond) and the node name in the drainage analysis, if applicable

2.83	20	A = Area draining to the practice	
0.44		A _I = Impervious area draining to the practice	
	decimal	I = percent impervious area draining to the practice, in decimal form	
	unitless	Rv = Runoff coefficient = 0.05 + (0.9 x I)	
	_		
	ac-in	WQV= 1" x Rv x A	
1,935	_	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
193	_	10% x WQV (check calc for sediment forebay and micropool volume)	
967		50% x WQV (check calc for extended detention volume)	1 > 400/W/OV
200	ct	V_{SED} = sediment forebay volume	$\leftarrow \geq 10\% \text{WQV}$
1,508	cf	V_{PP} = permanent pool volume (volume below the lowest invert of the ou	ıtlet structure) Attach
1,500		stage-storage table.	
yes	cf	Extended Detention? ¹	← ≤ 50% WQV
427		V_{ED} = Volume of Extended detention (if "yes is given in box above)	
500.15		E_{ED} = elevation of WQV if "yes" is given in box above ²	
0.01	cfs	$2Q_{avg} = 2*V_{ED} / 24 \text{ hrs } * (1 \text{hr} / 3600 \text{ sec}) \text{ (used to check against } Q_{EDmax}$	below)
0.01	cfs	Q_{EDmax} = discharge at the E_{ED} (attach stage-discharge table)	\leftarrow <2Q _{avg}
26.35	hours	T_{ED} = drawdown time of extended detention = $2V_{ED}/Q_{EDmax}$	← ≥ 24-hrs
3.00	:1	Pond side slopes	← ≥3:1
502.00	ft	Elevation of seasonal high water table	
500.00	ft	Elevation of lowest pond outlet	
497.00	ft	Max floor = maximum elevation of pond bottom (ft)	
492.00	ft	Minimum floor (to maintain depth at less than 8')	← ≤ 8 ft
107.00	_	3	\leftarrow \leq Max floor and $>$
497.00	_	Elevation of pond floor ³	← ≤ Max floor and > Min floor
497.00 75.00	ft	Elevation of pond floor ³ Length of the flow path between the inlet and outlet at mid-depth	
	ft ft	•	
75.00	ft ft ft	Length of the flow path between the inlet and outlet at mid-depth	
75.00 25.00	ft ft ft	Length of the flow path between the inlet and outlet at mid-depth Average Width ([average of the top width + average bottom width]/2)	Min floor
75.00 25.00 3.00	ft ft ft:1	Length of the flow path between the inlet and outlet at mid-depth Average Width ([average of the top width + average bottom width]/2) Length to Average Width ratio	Min floor
75.00 25.00 3.00 yes	ft ft ft :1 Yes/No	Length of the flow path between the inlet and outlet at mid-depth Average Width ([average of the top width + average bottom width]/2) Length to Average Width ratio The perimeter should be curvilinear.	Min floor ← ≥ 3:1
75.00 25.00 3.00 yes yes N	ft ft ft :1 Yes/No Yes/No	Length of the flow path between the inlet and outlet at mid-depth Average Width ([average of the top width + average bottom width]/2) Length to Average Width ratio The perimeter should be curvilinear. The inlet and outlet should be located as far apart as possible. Is there a manually-controlled drain to dewater the pond over a 24hr per	Min floor $\leftarrow \ge 3:1$
75.00 25.00 3.00 yes yes N	ft ft ft :1 Yes/No Yes/No Yes/No State why:	Length of the flow path between the inlet and outlet at mid-depth Average Width ([average of the top width + average bottom width]/2) Length to Average Width ratio The perimeter should be curvilinear. The inlet and outlet should be located as far apart as possible. Is there a manually-controlled drain to dewater the pond over a 24hr per What mechanism is proposed to prevent the outlet structure from cloggi	Min floor $\leftarrow \ge 3:1$
75.00 25.00 3.00 yes yes N	ft ft ft :1 Yes/No Yes/No Yes/No	Length of the flow path between the inlet and outlet at mid-depth Average Width ([average of the top width + average bottom width]/2) Length to Average Width ratio The perimeter should be curvilinear. The inlet and outlet should be located as far apart as possible. Is there a manually-controlled drain to dewater the pond over a 24hr per	Min floor $\leftarrow \ge 3:1$
75.00 25.00 3.00 yes yes N	ft ft ft :1 Yes/No Yes/No Yes/No o state why:	Length of the flow path between the inlet and outlet at mid-depth Average Width ([average of the top width + average bottom width]/2) Length to Average Width ratio The perimeter should be curvilinear. The inlet and outlet should be located as far apart as possible. Is there a manually-controlled drain to dewater the pond over a 24hr per What mechanism is proposed to prevent the outlet structure from cloggi	Min floor $\leftarrow \ge 3:1$
75.00 25.00 3.00 yes yes N If no NOT NEO 501.56 503.00	ft ft ft ft :1 Yes/No Yes/No Yes/No o state why: CESSARY ft	Length of the flow path between the inlet and outlet at mid-depth Average Width ([average of the top width + average bottom width]/2) Length to Average Width ratio The perimeter should be curvilinear. The inlet and outlet should be located as far apart as possible. Is there a manually-controlled drain to dewater the pond over a 24hr per What mechanism is proposed to prevent the outlet structure from cloggiorifices/weirs with a dimension of <6")? Peak elevation of the 50-year storm event Berm elevation of the pond	Min floor $\leftarrow \ge 3:1$
75.00 25.00 3.00 yes yes N If no	ft ft ft ft :1 Yes/No Yes/No Yes/No o state why: CESSARY ft	Length of the flow path between the inlet and outlet at mid-depth Average Width ([average of the top width + average bottom width]/2) Length to Average Width ratio The perimeter should be curvilinear. The inlet and outlet should be located as far apart as possible. Is there a manually-controlled drain to dewater the pond over a 24hr per What mechanism is proposed to prevent the outlet structure from cloggiorifices/weirs with a dimension of <6")? Peak elevation of the 50-year storm event	Min floor $\leftarrow \ge 3:1$

- 1. If the entire WQV is stored in the perm. pool, there is no extended det., and the following five lines do not apply.
- 2. This is the elevation of WQV if the hydrologic analysis is set up to include the permanent pool storage in the node description.
- 3. If the pond floor elevation is above the max floor elev., a hydrologic budget must be submitted to demonstrate that a minimum depth of 3 feet can be maintained. (First check whether a revised "lowest pond outlet" elev. will resolve the issue.)

Designer's Notes:

WETLANDS ON ALL SIDES

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 1

Stage-Discharge for Pond 1P: MICRO POOL 1

(feet) (cfs) (feet) (cfs) (feet) (cfs) 497.00 0.00 499.16 0.00 501.32 0.03 497.08 0.00 499.20 0.00 501.36 0.03 497.12 0.00 499.28 0.00 501.40 0.03 497.16 0.00 499.32 0.00 501.44 0.03 497.20 0.00 499.40 0.00 501.52 0.03 497.28 0.00 499.40 0.00 501.56 0.03 497.32 0.00 499.40 0.00 501.60 0.03 497.32 0.00 499.40 0.00 501.60 0.03 497.32 0.00 499.40 0.00 501.60 0.03 497.32 0.00 499.40 0.00 501.60 0.03 497.32 0.00 499.52 0.00 501.68 0.03 497.32 0.00 501.68 0.03 499.56 0.00 <t< th=""><th>Elevation</th><th>Primary</th><th>Elevation</th><th>Primary</th><th>Elevation</th><th>Primary</th></t<>	Elevation	Primary	Elevation	Primary	Elevation	Primary
497.04 0.00 499.24 0.00 501.36 0.03 497.12 0.00 499.28 0.00 501.44 0.03 497.16 0.00 499.32 0.00 501.48 0.03 497.20 0.00 499.36 0.00 501.52 0.03 497.24 0.00 499.40 0.00 501.52 0.03 497.28 0.00 499.44 0.00 501.60 0.03 497.32 0.00 499.44 0.00 501.64 0.03 497.36 0.00 499.52 0.00 501.68 0.03 497.40 0.00 499.56 0.00 501.72 0.03 497.44 0.00 499.60 0.00 501.76 0.03 497.50 0.00 499.64 0.00 501.88 0.04 497.50 0.00 499.72 0.00 501.88 0.04 497.60 0.00 499.80 0.00 501.92 0.04						(cfs)
497.08 0.00 499.24 0.00 501.40 0.03 497.16 0.00 499.32 0.00 501.48 0.03 497.20 0.00 499.36 0.00 501.52 0.03 497.24 0.00 499.40 0.00 501.56 0.03 497.28 0.00 499.44 0.00 501.64 0.03 497.32 0.00 499.48 0.00 501.64 0.03 497.36 0.00 499.52 0.00 501.68 0.03 497.40 0.00 499.56 0.00 501.76 0.03 497.44 0.00 499.60 0.00 501.76 0.03 497.48 0.00 499.64 0.00 501.80 0.03 497.50 0.00 499.72 0.00 501.80 0.03 497.60 0.00 499.72 0.00 501.88 0.04 497.64 0.00 499.84 0.00 502.00 0.04						
497.12 0.00 499.28 0.00 501.44 0.03 497.20 0.00 499.36 0.00 501.52 0.03 497.24 0.00 499.40 0.00 501.56 0.03 497.28 0.00 499.44 0.00 501.60 0.03 497.32 0.00 499.52 0.00 501.68 0.03 497.40 0.00 499.52 0.00 501.68 0.03 497.40 0.00 499.56 0.00 501.72 0.03 497.44 0.00 499.60 0.00 501.80 0.03 497.52 0.00 499.64 0.00 501.84 0.04 497.52 0.00 499.64 0.00 501.84 0.04 497.52 0.00 499.68 0.00 501.84 0.04 497.60 0.00 499.76 0.00 501.84 0.04 497.61 0.00 499.80 0.00 501.96 0.04						
497.16 0.00 499.32 0.00 501.48 0.03 497.24 0.00 499.40 0.00 501.52 0.03 497.28 0.00 499.44 0.00 501.60 0.03 497.32 0.00 499.44 0.00 501.68 0.03 497.36 0.00 499.52 0.00 501.68 0.03 497.40 0.00 499.56 0.00 501.72 0.03 497.44 0.00 499.66 0.00 501.72 0.03 497.48 0.00 499.64 0.00 501.80 0.03 497.52 0.00 499.68 0.00 501.84 0.04 497.50 0.00 499.76 0.00 501.84 0.04 497.60 0.00 499.80 0.00 501.96 0.04 497.61 0.00 499.84 0.00 502.00 0.04 497.62 0.00 499.84 0.00 502.00 0.04						
497.20 0.00 499.36 0.00 501.52 0.03 497.28 0.00 499.44 0.00 501.60 0.03 497.32 0.00 499.48 0.00 501.64 0.03 497.30 0.00 499.52 0.00 501.64 0.03 497.40 0.00 499.56 0.00 501.76 0.03 497.44 0.00 499.60 0.00 501.76 0.03 497.44 0.00 499.60 0.00 501.80 0.03 497.49 0.00 499.64 0.00 501.80 0.03 497.56 0.00 499.68 0.00 501.84 0.04 497.60 0.00 499.76 0.00 501.92 0.04 497.64 0.00 499.80 0.00 501.96 0.04 497.72 0.00 499.88 0.00 502.09 0.04 497.72 0.00 499.88 0.00 502.00 0.04						
497.24 0.00 499.40 0.00 501.56 0.03 497.28 0.00 499.44 0.00 501.60 0.03 497.36 0.00 499.52 0.00 501.68 0.03 497.40 0.00 499.56 0.00 501.72 0.03 497.44 0.00 499.60 0.00 501.76 0.03 497.48 0.00 499.64 0.00 501.80 0.03 497.52 0.00 499.68 0.00 501.80 0.03 497.48 0.00 499.68 0.00 501.84 0.04 497.52 0.00 499.68 0.00 501.84 0.04 497.50 0.00 499.72 0.00 501.98 0.04 497.60 0.00 499.80 0.00 501.96 0.04 497.61 0.00 499.84 0.00 501.96 0.04 497.76 0.00 499.84 0.00 502.00 0.04						
497.28 0.00 499.44 0.00 501.60 0.03 497.36 0.00 499.52 0.00 501.68 0.03 497.40 0.00 499.56 0.00 501.72 0.03 497.44 0.00 499.60 0.00 501.76 0.03 497.48 0.00 499.64 0.00 501.80 0.03 497.52 0.00 499.68 0.00 501.84 0.04 497.56 0.00 499.72 0.00 501.88 0.04 497.60 0.00 499.72 0.00 501.88 0.04 497.64 0.00 499.76 0.00 501.92 0.04 497.68 0.00 499.88 0.00 502.00 0.04 497.76 0.00 499.88 0.00 502.04 0.25 497.80 0.00 499.92 0.00 502.12 1.13 497.81 0.00 500.04 0.00 502.12 1.13						
497.36 0.00 499.52 0.00 501.68 0.03 497.44 0.00 499.60 0.00 501.72 0.03 497.48 0.00 499.64 0.00 501.80 0.03 497.52 0.00 499.68 0.00 501.84 0.04 497.56 0.00 499.72 0.00 501.88 0.04 497.60 0.00 499.72 0.00 501.88 0.04 497.64 0.00 499.80 0.00 501.96 0.04 497.68 0.00 499.84 0.00 502.00 0.04 497.77 0.00 499.88 0.00 502.00 0.02 497.76 0.00 499.92 0.00 502.12 1.13 497.84 0.00 500.00 0.00 502.12 1.13 497.84 0.00 500.00 0.00 502.22 2.38 497.84 0.00 500.00 0.00 502.12 1.17						
497.40 0.00 499.56 0.00 501.72 0.03 497.48 0.00 499.64 0.00 501.76 0.03 497.52 0.00 499.68 0.00 501.84 0.04 497.56 0.00 499.72 0.00 501.88 0.04 497.60 0.00 499.76 0.00 501.92 0.04 497.64 0.00 499.80 0.00 501.92 0.04 497.68 0.00 499.84 0.00 502.00 0.04 497.72 0.00 499.88 0.00 502.04 0.25 497.76 0.00 499.88 0.00 502.04 0.25 497.76 0.00 499.92 0.00 502.04 0.25 497.776 0.00 499.92 0.00 502.12 1.13 497.80 0.00 500.04 0.00 502.12 1.13 497.81 0.00 500.04 0.00 502.20 2.38						
497.44 0.00 499.60 0.00 501.76 0.03 497.48 0.00 499.68 0.00 501.80 0.03 497.52 0.00 499.72 0.00 501.88 0.04 497.60 0.00 499.76 0.00 501.92 0.04 497.64 0.00 499.80 0.00 501.96 0.04 497.68 0.00 499.84 0.00 502.00 0.04 497.72 0.00 499.88 0.00 502.00 0.04 497.76 0.00 499.88 0.00 502.00 0.04 497.77 0.00 499.88 0.00 502.08 0.63 497.80 0.00 499.96 0.00 502.12 1.13 497.81 0.00 500.00 0.00 502.12 1.13 497.82 0.00 500.04 0.00 502.20 2.38 497.92 0.00 500.08 0.01 502.22 2.38 497.92 0.00 500.08 0.01 502.24 3.11						
497.48 0.00 499.64 0.00 501.80 0.03 497.52 0.00 499.68 0.00 501.84 0.04 497.56 0.00 499.72 0.00 501.88 0.04 497.60 0.00 499.80 0.00 501.92 0.04 497.64 0.00 499.80 0.00 501.96 0.04 497.68 0.00 499.88 0.00 502.00 0.04 497.72 0.00 499.88 0.00 502.00 0.04 497.76 0.00 499.92 0.00 502.08 0.63 497.80 0.00 499.96 0.00 502.12 1.13 497.84 0.00 500.04 0.00 502.20 2.38 497.92 0.00 500.08 0.01 502.24 3.11 497.96 0.00 500.12 0.01 502.28 3.92 498.08 0.00 500.28 0.01 502.36 5.99						
497.52 0.00 499.68 0.00 501.84 0.04 497.66 0.00 499.72 0.00 501.88 0.04 497.60 0.00 499.76 0.00 501.92 0.04 497.64 0.00 499.80 0.00 501.96 0.04 497.68 0.00 499.84 0.00 502.00 0.04 497.76 0.00 499.88 0.00 502.00 0.04 497.76 0.00 499.92 0.00 502.08 0.63 497.76 0.00 499.92 0.00 502.08 0.63 497.80 0.00 500.00 502.12 1.13 497.84 0.00 500.00 500.01 502.20 2.38 497.92 0.00 500.08 0.01 502.24 3.11 497.96 0.00 500.12 0.01 502.28 3.92 498.00 0.00 500.16 0.01 502.32 4.78 49						
497.56 0.00 499.72 0.00 501.88 0.04 497.64 0.00 499.80 0.00 501.92 0.04 497.68 0.00 499.84 0.00 502.00 0.04 497.72 0.00 499.88 0.00 502.04 0.25 497.76 0.00 499.92 0.00 502.08 0.63 497.80 0.00 499.96 0.00 502.12 1.13 497.80 0.00 500.00 0.00 502.12 1.13 497.84 0.00 500.00 0.00 502.20 2.38 497.92 0.00 500.04 0.00 502.20 2.38 497.92 0.00 500.12 0.01 502.23 3.2 498.00 0.00 500.12 0.01 502.23 4.78 498.04 0.00 500.24 0.01 502.44 7.68 498.12 0.00 500.24 0.01 502.44 7.68						
497.60 0.00 499.76 0.00 501.92 0.04 497.64 0.00 499.80 0.00 501.96 0.04 497.68 0.00 499.84 0.00 502.00 0.04 497.72 0.00 499.92 0.00 502.08 0.63 497.80 0.00 499.96 0.00 502.12 1.13 497.84 0.00 500.00 0.00 502.16 1.71 497.88 0.00 500.04 0.00 502.20 2.38 497.92 0.00 500.08 0.01 502.22 2.38 497.92 0.00 500.08 0.01 502.23 3.92 498.00 0.00 500.12 0.01 502.23 3.92 498.04 0.00 500.20 0.01 502.32 4.78 498.16 0.00 500.28 0.01 502.44 7.68 498.20 0.00 500.32 0.01 502.52 9.85						
497.68 0.00 499.84 0.00 502.00 0.04 497.72 0.00 499.88 0.00 502.04 0.25 497.80 0.00 499.92 0.00 502.08 0.62 497.80 0.00 500.00 0.00 502.12 1.13 497.84 0.00 500.00 0.00 502.12 1.13 497.88 0.00 500.04 0.00 502.20 2.38 497.92 0.00 500.08 0.01 502.24 3.11 497.96 0.00 500.12 0.01 502.32 4.78 498.04 0.00 500.20 0.01 502.36 5.69 498.08 0.00 500.20 0.01 502.40 6.66 498.12 0.00 500.28 0.01 502.44 7.68 498.20 0.00 500.36 0.01 502.52 9.85 498.24 0.00 500.44 0.02 502.56 10.95			499.76			0.04
497.72 0.00 499.88 0.00 502.04 0.25 497.76 0.00 499.92 0.00 502.08 0.63 497.80 0.00 499.96 0.00 502.12 1.13 497.84 0.00 500.04 0.00 502.20 2.38 497.92 0.00 500.08 0.01 502.24 3.11 497.96 0.00 500.12 0.01 502.28 3.92 498.00 0.00 500.12 0.01 502.28 3.92 498.04 0.00 500.20 0.01 502.36 5.69 498.08 0.00 500.24 0.01 502.40 6.66 498.12 0.00 500.28 0.01 502.44 7.68 498.16 0.00 500.32 0.01 502.48 8.74 498.20 0.00 500.36 0.01 502.52 9.85 498.24 0.00 500.44 0.02 502.60 11.01						
497.76 0.00 499.92 0.00 502.08 0.63 497.80 0.00 499.96 0.00 502.12 1.13 497.84 0.00 500.00 0.00 502.16 1.71 497.88 0.00 500.08 0.01 502.20 2.38 497.92 0.00 500.08 0.01 502.24 3.11 497.96 0.00 500.12 0.01 502.28 3.92 498.04 0.00 500.20 0.01 502.32 4.78 498.04 0.00 500.24 0.01 502.32 4.78 498.08 0.00 500.24 0.01 502.36 6.66 498.12 0.00 500.28 0.01 502.44 7.68 498.16 0.00 500.32 0.01 502.48 8.74 498.24 0.00 500.40 0.02 502.52 9.85 498.28 0.00 500.44 0.02 502.66 11.01						
497.80 0.00 499.96 0.00 502.12 1.13 497.84 0.00 500.00 0.00 502.16 1.71 497.88 0.00 500.04 0.00 502.20 2.38 497.92 0.00 500.08 0.01 502.24 3.11 497.96 0.00 500.12 0.01 502.28 3.92 498.00 0.00 500.16 0.01 502.32 4.78 498.04 0.00 500.20 0.01 502.36 5.69 498.08 0.00 500.24 0.01 502.40 6.66 498.12 0.00 500.32 0.01 502.44 7.68 498.16 0.00 500.32 0.01 502.48 8.74 498.20 0.00 500.32 0.01 502.52 9.85 498.28 0.00 500.44 0.02 502.60 11.01 498.36 0.00 500.52 0.02 502.64 11.04						
497.84 0.00 500.00 0.00 502.16 1.71 497.88 0.00 500.04 0.00 502.20 2.38 497.92 0.00 500.08 0.01 502.24 3.11 497.96 0.00 500.12 0.01 502.28 3.92 498.00 0.00 500.16 0.01 502.32 4.78 498.04 0.00 500.20 0.01 502.36 5.69 498.08 0.00 500.24 0.01 502.40 6.66 498.12 0.00 500.28 0.01 502.44 7.68 498.16 0.00 500.32 0.01 502.48 8.74 498.20 0.00 500.36 0.01 502.52 9.85 498.24 0.00 500.40 0.02 502.56 10.95 498.28 0.00 500.44 0.02 502.60 11.01 498.36 0.00 500.48 0.02 502.68 11.41						
497.88 0.00 500.04 0.00 502.20 2.38 497.92 0.00 500.08 0.01 502.24 3.11 497.96 0.00 500.12 0.01 502.28 3.92 498.00 0.00 500.16 0.01 502.32 4.78 498.04 0.00 500.20 0.01 502.36 5.69 498.08 0.00 500.24 0.01 502.40 6.66 498.12 0.00 500.28 0.01 502.44 7.68 498.16 0.00 500.32 0.01 502.48 8.74 498.20 0.00 500.36 0.01 502.52 9.85 498.28 0.00 500.40 0.02 502.52 9.85 498.28 0.00 500.44 0.02 502.60 11.01 498.32 0.00 500.48 0.02 502.64 11.08 498.36 0.00 500.52 0.02 502.63 11.14						
497.96 0.00 500.12 0.01 502.28 3.92 498.00 0.00 500.16 0.01 502.32 4.78 498.04 0.00 500.20 0.01 502.36 5.69 498.08 0.00 500.24 0.01 502.40 6.66 498.12 0.00 500.32 0.01 502.44 7.68 498.16 0.00 500.32 0.01 502.48 8.74 498.20 0.00 500.36 0.01 502.52 9.85 498.24 0.00 500.40 0.02 502.56 10.95 498.38 0.00 500.44 0.02 502.60 11.01 498.32 0.00 500.48 0.02 502.64 11.08 498.36 0.00 500.52 0.02 502.64 11.04 498.36 0.00 500.60 0.02 502.72 11.20 498.44 0.00 500.60 0.02 502.80 11.33 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>						
498.00 0.00 500.16 0.01 502.32 4.78 498.04 0.00 500.20 0.01 502.36 5.69 498.08 0.00 500.24 0.01 502.40 6.66 498.12 0.00 500.28 0.01 502.44 7.68 498.16 0.00 500.32 0.01 502.48 8.74 498.20 0.00 500.36 0.01 502.52 9.85 498.24 0.00 500.40 0.02 502.56 10.95 498.28 0.00 500.44 0.02 502.60 11.01 498.32 0.00 500.48 0.02 502.60 11.01 498.36 0.00 500.52 0.02 502.64 11.08 498.40 0.00 500.56 0.02 502.72 11.20 498.44 0.00 500.60 0.02 502.72 11.27 498.48 0.00 500.64 0.02 502.80 11.33 498.52 0.00 500.68 0.02 502.84 11.39 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>						
498.04 0.00 500.20 0.01 502.36 5.69 498.08 0.00 500.24 0.01 502.40 6.66 498.12 0.00 500.28 0.01 502.44 7.68 498.16 0.00 500.32 0.01 502.48 8.74 498.20 0.00 500.36 0.01 502.52 9.85 498.24 0.00 500.40 0.02 502.56 10.95 498.28 0.00 500.44 0.02 502.60 11.01 498.36 0.00 500.48 0.02 502.64 11.08 498.36 0.00 500.52 0.02 502.64 11.08 498.40 0.00 500.52 0.02 502.72 11.20 498.44 0.00 500.60 0.02 502.72 11.27 498.48 0.00 500.64 0.02 502.80 11.33 498.52 0.00 500.68 0.02 502.84 11.39 498.60 0.00 500.76 0.02 502.92 11.51 <						
498.08 0.00 500.24 0.01 502.40 6.66 498.12 0.00 500.28 0.01 502.44 7.68 498.16 0.00 500.32 0.01 502.48 8.74 498.20 0.00 500.36 0.01 502.52 9.85 498.24 0.00 500.40 0.02 502.56 10.95 498.28 0.00 500.44 0.02 502.60 11.01 498.32 0.00 500.48 0.02 502.64 11.08 498.36 0.00 500.52 0.02 502.68 11.14 498.40 0.00 500.56 0.02 502.72 11.20 498.44 0.00 500.60 0.02 502.72 11.20 498.48 0.00 500.64 0.02 502.80 11.33 498.52 0.00 500.68 0.02 502.84 11.39 498.60 0.00 500.72 0.02 502.88 11.51 498.64 0.00 500.80 0.02 502.92 11.51						
498.12 0.00 500.28 0.01 502.44 7.68 498.16 0.00 500.32 0.01 502.48 8.74 498.20 0.00 500.36 0.01 502.52 9.85 498.24 0.00 500.40 0.02 502.56 10.95 498.28 0.00 500.44 0.02 502.60 11.01 498.32 0.00 500.48 0.02 502.64 11.08 498.36 0.00 500.52 0.02 502.68 11.14 498.40 0.00 500.56 0.02 502.72 11.20 498.44 0.00 500.60 0.02 502.76 11.27 498.48 0.00 500.64 0.02 502.80 11.33 498.52 0.00 500.68 0.02 502.84 11.39 498.60 0.00 500.72 0.02 502.88 11.45 498.64 0.00 500.80 0.02 502.92 11.51 498.68 0.00 500.84 0.02 503.00 11.63						
498.16 0.00 500.32 0.01 502.48 8.74 498.20 0.00 500.36 0.01 502.52 9.85 498.24 0.00 500.40 0.02 502.56 10.95 498.28 0.00 500.44 0.02 502.60 11.01 498.32 0.00 500.48 0.02 502.64 11.08 498.36 0.00 500.52 0.02 502.68 11.14 498.40 0.00 500.56 0.02 502.72 11.20 498.44 0.00 500.60 0.02 502.76 11.27 498.48 0.00 500.64 0.02 502.80 11.33 498.52 0.00 500.68 0.02 502.84 11.39 498.60 0.00 500.72 0.02 502.88 11.45 498.64 0.00 500.80 0.02 502.92 11.51 498.72 0.00 500.88 0.02 503.00 11.63 498.80 0.00 501.04 0.03 498.88 0.00						
498.20 0.00 500.36 0.01 502.52 9.85 498.24 0.00 500.40 0.02 502.56 10.95 498.28 0.00 500.44 0.02 502.60 11.01 498.32 0.00 500.48 0.02 502.64 11.08 498.36 0.00 500.52 0.02 502.68 11.14 498.40 0.00 500.56 0.02 502.72 11.20 498.44 0.00 500.60 0.02 502.76 11.27 498.48 0.00 500.64 0.02 502.80 11.33 498.52 0.00 500.68 0.02 502.84 11.39 498.56 0.00 500.72 0.02 502.88 11.45 498.60 0.00 500.76 0.02 502.92 11.51 498.64 0.00 500.80 0.02 502.96 11.57 498.68 0.00 500.88 0.02 503.00 11.63 498.72 0.00 500.88 0.02 503.00 11.63						
498.28 0.00 500.44 0.02 502.60 11.01 498.32 0.00 500.48 0.02 502.64 11.08 498.36 0.00 500.52 0.02 502.68 11.14 498.40 0.00 500.56 0.02 502.72 11.20 498.44 0.00 500.60 0.02 502.76 11.27 498.48 0.00 500.64 0.02 502.80 11.33 498.52 0.00 500.68 0.02 502.84 11.39 498.56 0.00 500.72 0.02 502.88 11.45 498.60 0.00 500.72 0.02 502.92 11.51 498.64 0.00 500.80 0.02 502.92 11.57 498.68 0.00 500.84 0.02 503.00 11.63 498.72 0.00 500.88 0.02 498.80 0.00 501.04 0.03 498.88 0.00 501.04 0.03 498.96 0.00 501.12 0.03 <						
498.32 0.00 500.48 0.02 502.64 11.08 498.36 0.00 500.52 0.02 502.68 11.14 498.40 0.00 500.56 0.02 502.72 11.20 498.44 0.00 500.60 0.02 502.76 11.27 498.48 0.00 500.64 0.02 502.80 11.33 498.52 0.00 500.68 0.02 502.84 11.39 498.56 0.00 500.72 0.02 502.84 11.39 498.60 0.00 500.72 0.02 502.92 11.51 498.64 0.00 500.80 0.02 502.92 11.57 498.68 0.00 500.84 0.02 502.96 11.57 498.68 0.00 500.88 0.02 503.00 11.63 498.76 0.00 500.92 0.02 498.80 0.00 501.00 0.03 498.88 0.00 501.04 0.03 0.03 498.92 0.00 501.12 0.03 499.04						
498.36 0.00 500.52 0.02 502.68 11.14 498.40 0.00 500.56 0.02 502.72 11.20 498.44 0.00 500.60 0.02 502.76 11.27 498.48 0.00 500.64 0.02 502.80 11.33 498.52 0.00 500.68 0.02 502.84 11.39 498.56 0.00 500.72 0.02 502.88 11.45 498.60 0.00 500.76 0.02 502.92 11.51 498.64 0.00 500.80 0.02 502.92 11.57 498.68 0.00 500.84 0.02 502.96 11.57 498.68 0.00 500.88 0.02 503.00 11.63 498.72 0.00 500.92 0.02 503.00 11.63 498.80 0.00 501.00 0.03 498.84 0.00 501.00 0.03 498.92 0.00 501.08 0.03 0.03 498.96 0.00 501.16 0.03 499.04						
498.40 0.00 500.56 0.02 502.72 11.20 498.44 0.00 500.60 0.02 502.76 11.27 498.48 0.00 500.64 0.02 502.80 11.33 498.52 0.00 500.68 0.02 502.84 11.39 498.56 0.00 500.72 0.02 502.88 11.45 498.60 0.00 500.76 0.02 502.92 11.51 498.64 0.00 500.80 0.02 502.92 11.57 498.68 0.00 500.84 0.02 502.96 11.57 498.68 0.00 500.88 0.02 503.00 11.63 498.72 0.00 500.88 0.02 503.00 11.63 498.80 0.00 500.92 0.02 498.80 0.00 501.00 0.03 498.84 0.00 501.04 0.03 0.03 498.92 0.00 501.08 0.03 498.96 0.00 501.12 0.03 0.03 499.04 0.00 501.20						
498.44 0.00 500.60 0.02 502.76 11.27 498.48 0.00 500.64 0.02 502.80 11.33 498.52 0.00 500.68 0.02 502.84 11.39 498.56 0.00 500.72 0.02 502.88 11.45 498.60 0.00 500.76 0.02 502.92 11.51 498.64 0.00 500.80 0.02 502.96 11.57 498.68 0.00 500.84 0.02 502.96 11.63 498.72 0.00 500.88 0.02 498.80 0.00 500.92 0.02 498.80 0.00 501.00 0.03 498.84 0.00 501.00 0.03 498.92 0.00 501.08 0.03 498.96 0.00 501.12 0.03 499.04 0.00 501.20 0.03 499.08 0.00 501.24 0.03						
498.48 0.00 500.64 0.02 502.80 11.33 498.52 0.00 500.68 0.02 502.84 11.39 498.56 0.00 500.72 0.02 502.88 11.45 498.60 0.00 500.76 0.02 502.92 11.51 498.64 0.00 500.80 0.02 502.96 11.57 498.68 0.00 500.84 0.02 503.00 11.63 498.72 0.00 500.88 0.02 503.00 11.63 498.80 0.00 500.92 0.02 498.80 0.00 500.92 0.02 498.84 0.00 501.00 0.03 498.88 0.00 501.04 0.03 498.92 0.00 501.08 0.03 498.96 0.00 501.12 0.03 499.04 0.00 501.20 0.03 499.04 0.00 501.20 0.03 499.08 0.00 501.24 0.03 0.03 0.03 0.03						
498.56 0.00 500.72 0.02 502.88 11.45 498.60 0.00 500.76 0.02 502.92 11.51 498.64 0.00 500.80 0.02 502.96 11.57 498.68 0.00 500.84 0.02 503.00 11.63 498.72 0.00 500.88 0.02 503.00 11.63 498.76 0.00 500.92 0.02 0.02 0.03 0						
498.60 0.00 500.76 0.02 502.92 11.51 498.64 0.00 500.80 0.02 502.96 11.57 498.68 0.00 500.84 0.02 503.00 11.63 498.72 0.00 500.88 0.02 498.76 0.00 500.92 0.02 498.80 0.00 500.92 0.03 0.03 0.03 0.03 498.84 0.00 501.00 0.03 0.03 0.03 0.03 0.03 498.92 0.00 501.08 0.03						
498.64 0.00 500.80 0.02 502.96 11.57 498.68 0.00 500.84 0.02 503.00 11.63 498.72 0.00 500.88 0.02 498.76 0.00 500.92 0.02 498.80 0.00 500.96 0.03 498.84 0.00 501.00 0.03 498.88 0.00 501.04 0.03 498.92 0.00 501.08 0.03 498.96 0.00 501.12 0.03 499.04 0.00 501.20 0.03 499.08 0.00 501.24 0.03						
498.68 0.00 500.84 0.02 503.00 11.63 498.72 0.00 500.88 0.02 498.76 0.00 500.92 0.02 498.80 0.00 500.96 0.03 498.84 0.00 501.00 0.03 498.88 0.00 501.04 0.03 498.92 0.00 501.08 0.03 498.96 0.00 501.12 0.03 499.00 499.00 501.16 0.03 499.04 0.00 501.20 0.03 499.08 0.00 501.24 0.03 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>						
498.72 0.00 500.88 0.02 498.76 0.00 500.92 0.02 498.80 0.00 500.96 0.03 498.84 0.00 501.00 0.03 498.88 0.00 501.04 0.03 498.92 0.00 501.08 0.03 498.96 0.00 501.12 0.03 499.00 0.00 501.16 0.03 499.04 0.00 501.20 0.03 499.08 0.00 501.24 0.03						
498.76 0.00 500.92 0.02 498.80 0.00 500.96 0.03 498.84 0.00 501.00 0.03 498.88 0.00 501.04 0.03 498.92 0.00 501.08 0.03 498.96 0.00 501.12 0.03 499.00 0.00 501.16 0.03 499.04 0.00 501.20 0.03 499.08 0.00 501.24 0.03					303.00	11.03
498.80 0.00 500.96 0.03 498.84 0.00 501.00 0.03 498.88 0.00 501.04 0.03 498.92 0.00 501.08 0.03 498.96 0.00 501.12 0.03 499.00 0.00 501.16 0.03 499.04 0.00 501.20 0.03 499.08 0.00 501.24 0.03						
498.88 0.00 501.04 0.03 498.92 0.00 501.08 0.03 498.96 0.00 501.12 0.03 499.00 0.00 501.16 0.03 499.04 0.00 501.20 0.03 499.08 0.00 501.24 0.03	498.80		500.96			
498.92 0.00 501.08 0.03 498.96 0.00 501.12 0.03 499.00 0.00 501.16 0.03 499.04 0.00 501.20 0.03 499.08 0.00 501.24 0.03						
498.96 0.00 501.12 0.03 499.00 0.00 501.16 0.03 499.04 0.00 501.20 0.03 499.08 0.00 501.24 0.03						
499.00 0.00 501.16 0.03 499.04 0.00 501.20 0.03 499.08 0.00 501.24 0.03						
499.04 0.00 501.20 0.03 499.08 0.00 501.24 0.03						
499.08 0.00 501.24 0.03						
499.12 0.00 501.28 0.03	499.08			0.03		
	499.12	0.00	501.28	0.03		

Storage

(cubic-feet) 10,457 10,925 11,402 11,888 12,382

12,886

13,398

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 2

Stage-Area-Storage for Pond 1P: MICRO POOL 1

Surface

(sq-ft) 4,643 4,728 4,814 4,900 4,988

5,076 5,165

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)
497.00	150	0	502.40
497.10	164	16	502.50
497.20	180	33	502.60
497.30	195	52	502.70
497.40	212	72	502.80
497.50	229	94	502.90
	247	118	
497.60			503.00
497.70	265	144	
497.80	285	171	
497.90	304	200	
498.00	325	232	
498.10	347	266	
498.20	370	301	
498.30	394	340	
498.40	419	380	
498.50	444	423	
498.60	470	469	
498.70	497	517	
498.80	524	568	
498.90	552	622	
499.00	581	679	
499.10	611	738	
499.20	641	801	
499.30	672	867	
499.40	704	936	
499.50	737	1,008	
499.60	770	1,083	
499.70	805	1,162	
499.80	839	1,244	
499.90	875	1,330	
500.00	2,895	1,508	
500.10	2,959	1,801	
500.20	3,024	2,100	
500.30	3,089	2,406	
500.40	3,156	2,718	
500.50	3,222	3,037	
500.60	3,290	3,363	
500.70	3,358	3,695	
500.80	3,427	4,034	
500.90	3,497	4,381	
501.00	3,567	4,734	
501.10	3,639	5,094	
501.20	3,710	5,461	
501.30	3,783	5,836	
501.40	3,856	6,218	
501.50	3,930	6,607	
501.60	4,005	7,004	
501.70	4,080	7,408	
501.80	4,156	7,400 7,820	
501.90	4,130	8,239	
502.00	4,233	8,667	
502.00	4,310	9,102	
502.10	4,392 4,475	9,102	
502.30	4,558	9,997	



STORMWATER POND DESIGN CRITERIA

Env-Wq 1508.03

Type/Node Name: WET POND 2 2-20-24

Enter the type of stormwater pond (e.g., Wet Pond) and the node name in the drainage analysis, if applicable

3.16	20	A = Area draining to the practice	
0.52	_	A_{I} = Impervious area draining to the practice	
	decimal	I = percent impervious area draining to the practice, in decimal form	
	unitless	Rv = Runoff coefficient = 0.05 + (0.9 x I)	
	_		
	ac-in	WQV= 1" x Rv x A	
2,272	_	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
227	_	10% x WQV (check calc for sediment forebay and micropool volume)	
1,136		50% x WQV (check calc for extended detention volume)	400/77/07/
OREBAY	cf	V_{SED} = sediment forebay volume	$\leftarrow \geq 10\% \text{WQV}$
250	cf	V_{PP} = permanent pool volume (volume below the lowest invert of the or	utlet structure) Attach
230	CI	stage-storage table.	
yes	cf	Extended Detention? ¹	← ≤ 50% WQV
2,022		V_{ED} = Volume of Extended detention (if "yes is given in box above)	
487.80		E_{ED} = elevation of WQV if "yes" is given in box above ²	
0.05	cfs	$2Q_{avg} = 2*V_{ED} / 24 \text{ hrs } * (1 \text{hr } / 3600 \text{ sec}) \text{ (used to check against } Q_{EDmax}$	below)
0.02	cfs	Q_{EDmax} = discharge at the E_{ED} (attach stage-discharge table)	← <2Q _{avg}
56.18	hours	T_{ED} = drawdown time of extended detention = $2V_{ED}/Q_{EDmax}$	$\leftarrow \ge 24$ -hrs
3.00	:1	Pond side slopes	← ≥3:1
490.00	ft	Elevation of seasonal high water table	
487.00	ft	Elevation of lowest pond outlet	
484.00	ft	Max floor = maximum elevation of pond bottom (ft)	
479.00	ft	Minimum floor (to maintain depth at less than 8')	← <u>≤</u> 8 ft
	_		\leftarrow \leq Max floor and $>$
484.00	ft	Elevation of pond floor ³	Min floor
54.00	ft	Length of the flow path between the inlet and outlet at mid-depth	
15.00	ft	Average Width ([average of the top width + average bottom width]/2)	
3.60		Length to Average Width ratio	← ≥ 3:1
yes	Yes/No	The perimeter should be curvilinear.	
yes	Yes/No	The inlet and outlet should be located as far apart as possible.	
N	Yes/No	Is there a manually-controlled drain to dewater the pond over a 24hr per	riod?
	state why:		
		What mechanism is proposed to prevent the outlet structure from cloggi	ing (applicable for
NOT NEO	CESSARY	orifices/weirs with a dimension of <6")?	
488.25	ft	Peak elevation of the 50-year storm event	
489.25	ft	Berm elevation of the pond	
YES		50 peak elevation ≤ the berm elevation?	← yes

- 1. If the entire WQV is stored in the perm. pool, there is no extended det., and the following five lines do not apply.
- 2. This is the elevation of WQV if the hydrologic analysis is set up to include the permanent pool storage in the node description.
- 3. If the pond floor elevation is above the max floor elev., a hydrologic budget must be submitted to demonstrate that a minimum depth of 3 feet can be maintained. (First check whether a revised "lowest pond outlet" elev. will resolve the issue.)

Designer's Notes:

WETLANDS ON ALL SIDES

Prepared by HP

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 1

Stage-Discharge for Pond 4P: MICRO POOL POND 2

Elevation	Primary	Elevation	Primary
(feet) 484.00	(cfs) 0.00	(feet) 486.70	(cfs) 0.00
484.05	0.00	486.75	0.00
484.10	0.00	486.80	0.00
484.15	0.00	486.85	0.00
484.20	0.00	486.90	0.00
484.25	0.00	486.95	0.00
484.30	0.00	487.00	0.00
484.35 484.40	0.00 0.00	487.05 487.10	0.00 0.01
484.45	0.00	487.15	0.01
484.50	0.00	487.20	0.01
484.55	0.00	487.25	0.01
484.60	0.00	487.30	0.01
484.65 484.70	0.00 0.00	487.35 487.40	0.01 0.02
484.75	0.00	487.45	0.02
484.80	0.00	487.50	0.02
484.85	0.00	487.55	0.02
484.90	0.00	487.60	0.02
484.95	0.00	487.65	0.02
485.00 485.05	0.00 0.00	487.70 487.75	0.02 0.02
485.10	0.00	487.80	0.02
485.15	0.00	487.85	0.02
485.20	0.00	487.90	0.02
485.25	0.00	487.95	0.03
485.30	0.00	488.00	0.03
485.35 485.40	0.00 0.00	488.05 488.10	0.03 0.03
485.45	0.00	488.15	0.03
485.50	0.00	488.20	0.03
485.55	0.00	488.25	0.03
485.60	0.00	488.30	0.32
485.65 485.70	0.00	488.35 488.40	0.86
485.75	0.00 0.00	488.45	1.55 2.37
485.80	0.00	488.50	3.30
485.85	0.00	488.55	4.33
485.90	0.00	488.60	5.45
485.95	0.00	488.65	6.65
486.00 486.05	0.00 0.00	488.70 488.75	7.93 9.28
486.10	0.00	488.80	10.71
486.15	0.00	488.85	10.94
486.20	0.00	488.90	11.01
486.25	0.00	488.95	11.09
486.30	0.00	489.00	11.17
486.35 486.40	0.00 0.00		
486.45	0.00		
486.50	0.00		
486.55	0.00		
486.60	0.00		
486.65	0.00		

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 2

Stage-Area-Storage for Pond 4P: MICRO POOL POND 2

Elevation Surface (feet) (sq.ft) (cubic-feet) (feet) (sq.ft) (cubic-feet) (feet) (sq.ft) (cubic-feet) (feet) (sq.ft) (cubic-feet) (sq.ft) (cubic-feet) (sq.ft) (sq.ft) (cubic-feet) (sq.ft) (sq.ft						
## 484.00						
## 484.05						
484.10 99 9 486.80 718 1,011 484.15 108 14 486.85 734 1,047 484.20 118 20 486.90 750 1,084 484.25 128 26 486.95 900 1,126 484.30 137 33 487.00 1,061 1,175 484.35 147 40 487.05 1,080 1,228 484.40 156 47 487.10 1,109 1,283 484.45 165 55 487.15 1,138 1,339 484.50 175 64 487.20 1,167 1,396 484.55 185 73 487.25 1,197 1,455 484.60 194 82 487.30 1,226 1,516 484.65 203 92 487.35 1,255 1,578 484.80 232 123 103 487.45 1,313 1,706 484.80 <td></td> <td></td> <td></td> <td></td> <td></td> <td>-</td>						-
484.15 108 14 486.85 734 1,047 484.20 118 20 486.90 750 1,084 484.25 128 26 486.95 900 1,126 484.30 137 33 487.00 1,081 1,175 484.40 156 47 487.10 1,109 1,283 484.45 165 55 487.15 1,138 1,339 484.50 175 64 487.20 1,167 1,396 484.60 194 82 487.30 1,226 1,516 484.65 203 92 487.35 1,255 1,578 484.70 213 103 487.45 1,313 1,706 484.80 232 125 487.50 1,342 1,773 484.85 242 137 487.55 1,371 1,841 484.90 251 149 487.60 1,400 1,910 485.05 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td></td<>						
## ## ## ## ## ## ## ## ## ## ## ## ##						
## ## ## ## ## ## ## ## ## ## ## ## ##						
484.30 137 33 487.00 1,051 1,175 484.35 147 40 487.05 1,080 1,228 484.40 156 47 487.10 1,109 1,283 484.55 165 55 487.15 1,138 1,339 484.55 185 73 487.20 1,167 1,396 484.60 194 82 487.30 1,226 1,516 484.65 203 92 487.35 1,255 1,578 484.70 213 103 487.40 1,284 1,642 484.75 223 113 487.45 1,313 1,706 484.80 232 125 487.50 1,342 1,773 484.80 232 125 487.50 1,342 1,773 484.80 232 137 487.55 1,371 1,841 484.90 251 149 487.60 1,400 1,910 484.95						
484.35 147 40 487.05 1,080 1,228 484.40 156 47 487.10 1,109 1,228 484.45 165 55 487.15 1,138 1,339 484.50 175 64 487.20 1,167 1,396 484.60 194 82 487.30 1,226 1,516 484.60 194 82 487.30 1,226 1,516 484.61 213 103 487.40 1,225 1,516 484.75 223 113 487.45 1,313 1,706 484.80 232 125 487.50 1,342 1,773 484.80 232 125 487.50 1,342 1,773 484.80 232 125 487.55 1,371 1,841 484.90 251 149 487.60 1,400 1,910 484.95 260 162 487.65 1,429 1,981 485.05						
484.40 156 47 487.10 1,109 1,283 484.45 165 55 54 487.15 1,138 1,339 484.50 175 64 487.20 1,167 1,396 484.55 185 73 487.25 1,197 1,455 484.60 194 82 487.30 1,226 1,516 484.65 203 92 487.35 1,255 1,578 484.70 213 103 487.40 1,284 1,642 484.75 223 113 487.45 1,313 1,706 484.80 232 125 487.50 1,342 1,773 484.85 242 137 487.55 1,371 1,841 484.90 251 149 487.60 1,400 1,910 484.95 260 162 487.65 1,429 1,981 485.00 270 175 487.70 1,458 2,053						
484.45 165 55 487.15 1,138 1,339 484.50 175 64 487.20 1,167 1,396 484.55 185 73 487.25 1,197 1,455 484.60 194 82 487.35 1,226 1,516 484.65 203 92 487.35 1,225 1,518 484.70 213 103 487.40 1,284 1,642 484.80 232 125 487.50 1,342 1,773 484.85 242 137 487.55 1,371 1,841 484.90 251 149 487.60 1,400 1,911 485.00 270 175 487.65 1,429 1,981 485.05 280 189 487.75 1,488 2,126 485.10 289 203 487.80 1,517 2,202 485.10 289 203 487.85 1,546 2,278 485.25						1,220
484.50 175 64 487.20 1,167 1,396 484.55 185 73 487.25 1,197 1,455 484.60 194 82 487.30 1,226 1,516 484.65 203 92 487.35 1,255 1,578 484.70 213 103 487.40 1,284 1,642 484.75 223 113 487.45 1,313 1,706 484.80 232 125 487.50 1,342 1,773 484.85 242 137 487.55 1,371 1,841 484.90 251 149 487.60 1,400 1,910 484.95 260 162 487.65 1,429 1,981 485.00 270 175 487.70 1,458 2,053 485.05 280 189 487.75 1,488 2,126 485.15 298 203 487.80 1,517 2,202 485.25						
484.55 185 73 487.25 1,197 1,455 484.60 194 82 487.30 1,226 1,516 484.65 203 92 487.35 1,255 1,578 484.70 213 103 487.40 1,284 1,642 484.75 223 113 487.45 1,313 1,706 484.80 232 125 487.50 1,342 1,773 484.85 242 137 487.55 1,371 1,841 484.90 251 149 487.60 1,400 1,910 484.95 260 162 487.65 1,429 1,981 485.00 270 175 487.70 1,458 2,053 485.10 289 203 487.80 1,517 2,202 485.15 298 218 487.85 1,546 2,278 485.20 308 233 487.90 1,575 2,356 485.30						
484.60 194 82 487.30 1,226 1,516 484.65 203 92 487.35 1,255 1,578 484.70 213 103 487.40 1,284 1,642 484.85 223 113 487.45 1,313 1,706 484.85 242 137 487.55 1,371 1,841 484.90 251 149 487.60 1,400 1,910 484.95 260 162 487.65 1,429 1,981 485.00 270 175 487.70 1,458 2,053 485.05 280 189 487.75 1,488 2,126 485.10 289 203 487.80 1,517 2,202 485.15 298 218 487.85 1,546 2,278 485.20 308 233 487.90 1,575 2,356 485.25 318 248 487.95 1,604 2,436 485.35 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>						
484.65 203 92 487.35 1,255 1,578 484.70 213 103 487.40 1,284 1,642 484.75 223 113 487.45 1,313 1,706 484.80 232 125 487.50 1,342 1,773 484.85 242 137 487.55 1,371 1,841 484.90 251 149 487.60 1,400 1,910 484.95 260 162 487.65 1,429 1,981 485.00 270 175 487.70 1,458 2,053 485.05 280 189 487.75 1,488 2,126 485.10 289 203 487.80 1,517 2,202 485.21 298 218 487.85 1,546 2,278 485.25 318 248 487.95 1,604 2,436 485.30 327 265 488.00 1,633 2,517 485.45 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>						
484.70 213 103 487.40 1,284 1,642 484.75 223 113 487.45 1,313 1,706 484.80 232 125 487.50 1,342 1,773 484.85 242 137 487.55 1,371 1,841 484.90 251 149 487.60 1,400 1,910 484.95 260 162 487.65 1,429 1,981 485.00 270 175 487.70 1,458 2,053 485.05 280 189 487.75 1,488 2,126 485.10 289 203 487.80 1,517 2,202 485.15 298 218 487.85 1,546 2,278 485.20 308 233 487.90 1,575 2,356 485.30 327 265 488.00 1,633 2,517 485.35 337 281 488.05 1,666 2,599 485.45 355 316 488.15 1,731 2,769 485.50						1,510
484.75 223 113 487.45 1,313 1,706 484.80 232 125 487.50 1,342 1,773 484.85 242 137 487.55 1,371 1,841 484.90 251 149 487.60 1,400 1,910 484.95 260 162 487.65 1,429 1,981 485.00 270 175 487.70 1,458 2,053 485.05 280 189 487.75 1,488 2,126 485.10 289 203 487.80 1,517 2,202 485.15 298 218 487.85 1,546 2,278 485.20 308 233 487.90 1,575 2,356 485.30 327 265 488.00 1,633 2,517 485.35 337 281 488.05 1,604 2,436 485.45 355 316 488.15 1,731 2,769 485.50 365 334 488.25 1,797 2,945 485.60						
484.80 232 125 487.50 1,342 1,773 484.85 242 137 487.55 1,371 1,841 484.90 251 149 487.60 1,400 1,910 484.95 260 162 487.65 1,429 1,981 485.00 270 175 487.70 1,458 2,053 485.05 280 189 487.75 1,488 2,126 485.10 289 203 487.80 1,517 2,202 485.15 298 218 487.85 1,546 2,278 485.20 308 233 487.90 1,575 2,356 485.30 327 265 488.00 1,633 2,517 485.35 337 281 488.05 1,666 2,599 485.40 346 298 488.10 1,699 2,683 485.50 365 334 488.20 1,764 2,856 485.50 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>						
484.85 242 137 487.55 1,371 1,841 484.90 251 149 487.60 1,400 1,910 484.95 260 162 487.65 1,429 1,981 485.00 270 175 487.70 1,458 2,053 485.05 280 189 487.75 1,488 2,126 485.10 289 203 487.80 1,517 2,202 485.15 298 218 487.85 1,546 2,278 485.20 308 233 487.90 1,575 2,356 485.25 318 248 487.95 1,604 2,436 485.30 327 265 488.00 1,633 2,517 485.35 337 281 488.05 1,666 2,599 485.40 346 298 488.10 1,699 2,683 485.55 355 316 488.15 1,731 2,769 485.50 365 334 488.20 1,764 2,856 485.60						
484.90 251 149 487.60 1,400 1,910 484.95 260 162 487.65 1,429 1,981 485.00 270 175 487.70 1,458 2,053 485.05 280 189 487.75 1,488 2,126 485.10 289 203 487.80 1,517 2,202 485.15 298 218 487.85 1,546 2,278 485.20 308 233 487.90 1,575 2,356 485.25 318 248 487.95 1,604 2,436 485.30 327 265 488.00 1,633 2,517 485.45 355 337 281 488.05 1,666 2,599 485.45 355 316 488.15 1,731 2,769 485.50 365 334 488.20 1,764 2,856 485.65 375 352 488.25 1,797 2,945						
484.95 260 162 487.65 1,429 1,981 485.00 270 175 487.70 1,458 2,053 485.05 280 189 487.75 1,488 2,126 485.10 289 203 487.80 1,517 2,202 485.15 298 218 487.85 1,546 2,278 485.20 308 233 487.90 1,575 2,356 485.25 318 248 487.95 1,604 2,436 485.30 327 265 488.00 1,633 2,517 485.35 337 281 488.05 1,666 2,599 485.40 346 298 488.10 1,699 2,683 485.50 365 334 488.20 1,764 2,856 485.55 375 352 488.25 1,797 2,945 485.60 384 371 488.30 1,862 3,128 485.75 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>						
485.00 270 175 487.70 1,458 2,053 485.05 280 189 487.75 1,488 2,126 485.10 289 203 487.80 1,517 2,202 485.15 298 218 487.85 1,546 2,278 485.20 308 233 487.90 1,575 2,356 485.25 318 248 487.95 1,604 2,436 485.30 327 265 488.00 1,633 2,517 485.35 337 281 488.05 1,666 2,599 485.40 346 298 488.10 1,699 2,683 485.55 355 316 488.15 1,731 2,769 485.55 375 352 488.25 1,797 2,945 485.60 384 371 488.30 1,830 3,036 485.75 413 431 488.40 1,928 3,318 485.70 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>						
485.05 280 189 487.75 1,488 2,126 485.10 289 203 487.80 1,517 2,202 485.15 298 218 487.85 1,546 2,278 485.20 308 233 487.90 1,575 2,356 485.25 318 248 487.95 1,604 2,436 485.30 327 265 488.00 1,633 2,517 485.35 337 281 488.05 1,666 2,599 485.40 346 298 488.15 1,731 2,769 485.45 355 316 488.15 1,731 2,769 485.50 365 334 488.20 1,764 2,856 485.55 375 352 488.25 1,797 2,945 485.60 384 371 488.30 1,830 3,036 485.65 393 391 488.40 1,895 3,222 485.75 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>						
485.10 289 203 487.80 1,517 2,202 485.15 298 218 487.85 1,546 2,278 485.20 308 233 487.90 1,575 2,356 485.25 318 248 487.95 1,604 2,436 485.30 327 265 488.00 1,633 2,517 485.35 337 281 488.05 1,666 2,599 485.40 346 298 488.10 1,699 2,683 485.45 355 316 488.15 1,731 2,769 485.50 365 334 488.20 1,764 2,856 485.60 384 371 488.30 1,830 3,036 485.65 393 391 488.35 1,862 3,128 485.70 403 411 488.40 1,935 3,222 485.75 413 431 488.45 1,928 3,318 485.80 </td <td></td> <td>280</td> <td></td> <td></td> <td></td> <td></td>		280				
485.20 308 233 487.90 1,575 2,356 485.25 318 248 487.95 1,604 2,436 485.30 327 265 488.00 1,633 2,517 485.35 337 281 488.05 1,666 2,599 485.40 346 298 488.10 1,699 2,683 485.45 355 316 488.15 1,731 2,769 485.50 365 334 488.20 1,764 2,856 485.55 375 352 488.25 1,797 2,945 485.60 384 371 488.30 1,830 3,036 485.65 393 391 488.35 1,862 3,128 485.75 413 431 488.45 1,928 3,318 485.80 422 452 488.50 1,961 3,415 485.85 432 473 488.55 1,993 3,514 485.95 450 517 488.65 2,059 3,716 486.00	485.10	289	203	487.80		
485.25 318 248 487.95 1,604 2,436 485.30 327 265 488.00 1,633 2,517 485.35 337 281 488.05 1,666 2,599 485.40 346 298 488.10 1,699 2,683 485.45 355 316 488.15 1,731 2,769 485.50 365 334 488.20 1,764 2,856 485.55 375 352 488.25 1,797 2,945 485.60 384 371 488.30 1,830 3,036 485.65 393 391 488.35 1,862 3,128 485.70 403 411 488.40 1,895 3,222 485.75 413 431 488.45 1,928 3,318 485.80 422 452 488.50 1,961 3,415 485.85 432 473 488.55 1,993 3,514 485.90 </td <td>485.15</td> <td>298</td> <td>218</td> <td>487.85</td> <td>1,546</td> <td>2,278</td>	485.15	298	218	487.85	1,546	2,278
485.30 327 265 488.00 1,633 2,517 485.35 337 281 488.05 1,666 2,599 485.40 346 298 488.10 1,699 2,683 485.45 355 316 488.15 1,731 2,769 485.50 365 334 488.20 1,764 2,856 485.55 375 352 488.25 1,797 2,945 485.60 384 371 488.30 1,830 3,036 485.65 393 391 488.35 1,862 3,128 485.70 403 411 488.40 1,895 3,222 485.75 413 431 488.45 1,928 3,318 485.80 422 452 488.50 1,961 3,415 485.85 432 473 488.55 1,993 3,514 485.90 441 495 488.60 2,026 3,614 485.95 450 517 488.65 2,059 3,716 486.00	485.20			487.90	1,575	2,356
485.35 337 281 488.05 1,666 2,599 485.40 346 298 488.10 1,699 2,683 485.45 355 316 488.15 1,731 2,769 485.50 365 334 488.20 1,764 2,856 485.55 375 352 488.25 1,797 2,945 485.60 384 371 488.30 1,830 3,036 485.65 393 391 488.35 1,862 3,128 485.70 403 411 488.40 1,895 3,222 485.75 413 431 488.45 1,928 3,318 485.80 422 452 488.50 1,961 3,415 485.85 432 473 488.55 1,993 3,514 485.90 441 495 488.60 2,026 3,614 485.95 450 517 488.65 2,059 3,716 486.00 460 540 488.70 2,091 3,820 486.15						
485.40 346 298 488.10 1,699 2,683 485.45 355 316 488.15 1,731 2,769 485.50 365 334 488.20 1,764 2,856 485.55 375 352 488.25 1,797 2,945 485.60 384 371 488.30 1,830 3,036 485.65 393 391 488.35 1,862 3,128 485.70 403 411 488.40 1,895 3,222 485.75 413 431 488.45 1,928 3,318 485.80 422 452 488.50 1,961 3,415 485.85 432 473 488.55 1,993 3,514 485.90 441 495 488.60 2,026 3,614 485.95 450 517 488.65 2,059 3,716 486.05 476 563 488.75 2,124 3,926 486.10 492 588 488.80 2,157 4,033 486.25						
485.45 355 316 488.15 1,731 2,769 485.50 365 334 488.20 1,764 2,856 485.55 375 352 488.25 1,797 2,945 485.60 384 371 488.30 1,830 3,036 485.65 393 391 488.35 1,862 3,128 485.70 403 411 488.40 1,895 3,222 485.75 413 431 488.45 1,928 3,318 485.80 422 452 488.50 1,961 3,415 485.85 432 473 488.55 1,993 3,514 485.90 441 495 488.60 2,026 3,614 485.95 450 517 488.65 2,059 3,716 486.00 460 540 488.70 2,091 3,820 486.10 492 588 488.80 2,157 4,033 486.15 508 613 488.85 2,190 4,141 486.25						
485.50 365 334 488.20 1,764 2,856 485.55 375 352 488.25 1,797 2,945 485.60 384 371 488.30 1,830 3,036 485.65 393 391 488.35 1,862 3,128 485.70 403 411 488.40 1,895 3,222 485.75 413 431 488.45 1,928 3,318 485.80 422 452 488.50 1,961 3,415 485.85 432 473 488.55 1,993 3,514 485.90 441 495 488.60 2,026 3,614 485.95 450 517 488.65 2,059 3,716 486.00 460 540 488.70 2,091 3,820 486.05 476 563 488.75 2,124 3,926 486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.35						
485.55 375 352 488.25 1,797 2,945 485.60 384 371 488.30 1,830 3,036 485.65 393 391 488.35 1,862 3,128 485.70 403 411 488.40 1,895 3,222 485.75 413 431 488.45 1,928 3,318 485.80 422 452 488.50 1,961 3,415 485.85 432 473 488.55 1,993 3,514 485.90 441 495 488.60 2,026 3,614 485.95 450 517 488.65 2,059 3,716 486.00 460 540 488.70 2,091 3,820 486.05 476 563 488.75 2,124 3,926 486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.35 573 721 486.45 605 780 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td></t<>						
485.60 384 371 488.30 1,830 3,036 485.65 393 391 488.35 1,862 3,128 485.70 403 411 488.40 1,895 3,222 485.75 413 431 488.45 1,928 3,318 485.80 422 452 488.50 1,961 3,415 485.85 432 473 488.55 1,993 3,514 485.90 441 495 488.60 2,026 3,614 485.95 450 517 488.65 2,059 3,716 486.00 460 540 488.70 2,091 3,820 486.05 476 563 488.75 2,124 3,926 486.10 492 588 488.80 2,157 4,033 486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.35 573 721 486.40 589 750 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td></t<>						
485.65 393 391 488.35 1,862 3,128 485.70 403 411 488.40 1,895 3,222 485.75 413 431 488.45 1,928 3,318 485.80 422 452 488.50 1,961 3,415 485.85 432 473 488.55 1,993 3,514 485.90 441 495 488.60 2,026 3,614 485.95 450 517 488.65 2,059 3,716 486.00 460 540 488.70 2,091 3,820 486.05 476 563 488.75 2,124 3,926 486.10 492 588 488.80 2,157 4,033 486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.30 557 693 489.00 2,288 4,477 486.40 589 750 486.45 605 780 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td></t<>						
485.70 403 411 488.40 1,895 3,222 485.75 413 431 488.45 1,928 3,318 485.80 422 452 488.50 1,961 3,415 485.85 432 473 488.55 1,993 3,514 485.90 441 495 488.60 2,026 3,614 485.95 450 517 488.65 2,059 3,716 486.00 460 540 488.70 2,091 3,820 486.05 476 563 488.75 2,124 3,926 486.10 492 588 488.80 2,157 4,033 486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.30 557 693 489.00 2,288 4,477 486.40 589 750 486.55 637 842 486.55 637 842 486.60 653 874						3,036
485.75 413 431 488.45 1,928 3,318 485.80 422 452 488.50 1,961 3,415 485.85 432 473 488.55 1,993 3,514 485.90 441 495 488.60 2,026 3,614 485.95 450 517 488.65 2,059 3,716 486.00 460 540 488.70 2,091 3,820 486.05 476 563 488.75 2,124 3,926 486.10 492 588 488.80 2,157 4,033 486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.25 541 665 488.95 2,255 4,363 486.30 557 693 489.00 2,288 4,477 486.45 605 780 486.55 637 842 486.60 653 874						
485.80 422 452 488.50 1,961 3,415 485.85 432 473 488.55 1,993 3,514 485.90 441 495 488.60 2,026 3,614 485.95 450 517 488.65 2,059 3,716 486.00 460 540 488.70 2,091 3,820 486.05 476 563 488.75 2,124 3,926 486.10 492 588 488.80 2,157 4,033 486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.25 541 665 488.95 2,255 4,363 486.30 557 693 489.00 2,288 4,477 486.45 605 780 486.55 637 842 486.55 637 842 486.60 653 874						
485.85 432 473 488.55 1,993 3,514 485.90 441 495 488.60 2,026 3,614 485.95 450 517 488.65 2,059 3,716 486.00 460 540 488.70 2,091 3,820 486.05 476 563 488.75 2,124 3,926 486.10 492 588 488.80 2,157 4,033 486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.25 541 665 488.95 2,255 4,363 486.30 557 693 489.00 2,288 4,477 486.40 589 750 486.45 605 780 486.55 637 842 486.60 653 874						
485.90 441 495 488.60 2,026 3,614 485.95 450 517 488.65 2,059 3,716 486.00 460 540 488.70 2,091 3,820 486.05 476 563 488.75 2,124 3,926 486.10 492 588 488.80 2,157 4,033 486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.25 541 665 488.95 2,255 4,363 486.30 557 693 489.00 2,288 4,477 486.40 589 750 486.45 605 780 486.55 637 842 486.60 653 874						
485.95 450 517 488.65 2,059 3,716 486.00 460 540 488.70 2,091 3,820 486.05 476 563 488.75 2,124 3,926 486.10 492 588 488.80 2,157 4,033 486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.25 541 665 488.95 2,255 4,363 486.30 557 693 489.00 2,288 4,477 486.40 589 750 486.45 605 780 486.50 621 810 486.55 637 842 486.60 653 874						
486.00 460 540 488.70 2,091 3,820 486.05 476 563 488.75 2,124 3,926 486.10 492 588 488.80 2,157 4,033 486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.25 541 665 488.95 2,255 4,363 486.30 557 693 489.00 2,288 4,477 486.35 573 721 486.40 589 750 486.45 605 780 486.50 621 810 486.55 637 842 486.60 653 874						
486.05 476 563 488.75 2,124 3,926 486.10 492 588 488.80 2,157 4,033 486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.25 541 665 488.95 2,255 4,363 486.30 557 693 489.00 2,288 4,477 486.35 573 721 486.40 589 750 486.45 605 780 486.50 621 810 486.55 637 842 486.60 653 874						
486.10 492 588 488.80 2,157 4,033 486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.25 541 665 488.95 2,255 4,363 486.30 557 693 489.00 2,288 4,477 486.35 573 721 486.40 589 750 486.45 605 780 486.50 621 810 486.55 637 842 486.60 653 874						
486.15 508 613 488.85 2,190 4,141 486.20 524 638 488.90 2,222 4,252 486.25 541 665 488.95 2,255 4,363 486.30 557 693 489.00 2,288 4,477 486.35 573 721 486.40 589 750 486.45 605 780 486.50 621 810 486.55 637 842 486.60 653 874						
486.20 524 638 488.90 2,222 4,252 486.25 541 665 488.95 2,255 4,363 486.30 557 693 489.00 2,288 4,477 486.35 573 721 486.40 589 750 486.45 605 780 486.50 621 810 486.55 637 842 486.60 653 874						
486.25 541 665 488.95 2,255 4,363 486.30 557 693 489.00 2,288 4,477 486.35 573 721 486.40 589 750 486.45 605 780 486.50 621 810 486.55 637 842 486.60 653 874						
486.35 573 721 486.40 589 750 486.45 605 780 486.50 621 810 486.55 637 842 486.60 653 874	486.25	541	665	488.95		
486.40 589 750 486.45 605 780 486.50 621 810 486.55 637 842 486.60 653 874	486.30	557	693	489.00	2,288	4,477
486.45 605 780 486.50 621 810 486.55 637 842 486.60 653 874	486.35		721			
486.50 621 810 486.55 637 842 486.60 653 874						
486.55 637 842 486.60 653 874						
486.60 653 874						
486.65 669 907						
	486.65	669	907			

OUTLET PROTECTION

RIPRAP CALCULATIONS DESIGN STORM: 25 YEARS

DATE: 2/20/24 REVISED:

PROJECT NAME: KEARSARGE ROAD

LOCATION: WARNER JOB NO.

VARIABLES:

Q = DISCHARGE FROM OUTLET

Do = PIPE DIAMETER Tw = TAIL WATER

La = LENGTH OF RIPRAP

Wi = WIDTH OF RIPRAP AT OUTLET

We = WIDTH OF RIPRAP DOWNSTREAM FROM OUTLET

d50 = RIPRAP SIZE

EQUATIONS:

FOR Tw < 1/2 Do

FOR Tw > or = 1/2 Do

 $La = (1.8Q/Do^{1.5}) + 7Do$ $La = (3Q)/(Do^{1.5})$

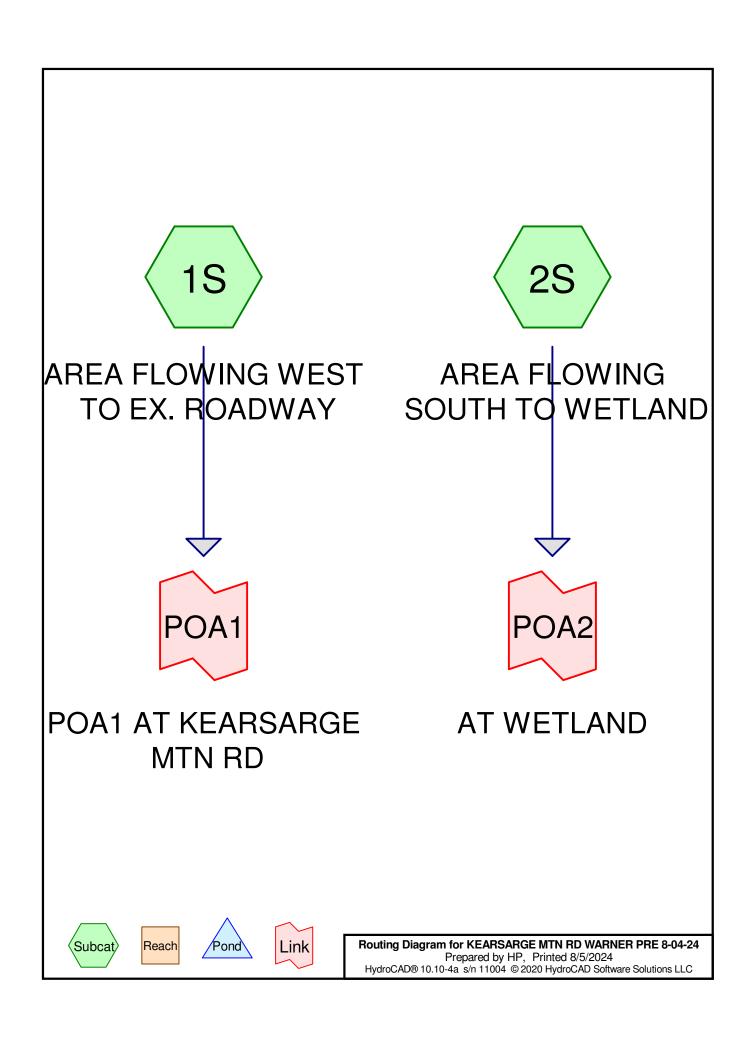
Wi = 3Do Wi = 3Do

We = Do + La We = Do + 0.4La

 $d50 = (0.02Q^{1.33})/(Tw)(Do)$ $d50 = (0.02Q^{1.33})/(Tw)(Do)$

OUTLET	Q CFS	Do FEET	Tw FEET	La FEET	Wi FEET	We FEET	d50 INCHES
FES 1	0.72	1.25	0.10	9.68	3.75	10.93	1.24
FES 2	0.02	1.25	0.10	8.78	3.75	10.03	0.01

PREDEVELOPMENT DRAINAGE CALCULATIONS



Prepared by HP
HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Printed 8/5/2024 Page 2

Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
3.333	39	>75% Grass cover, Good, HSG A (1S, 2S)
0.046	98	Existing roadway, HSG A (1S)
0.038	98	Paved parking, HSG A (1S)
0.029	98	Roofs, HSG A (1S)
25.449	30	Woods, Good, HSG A (1S, 2S)
28.895	31	TOTAL AREA

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Printed 8/5/2024 Page 3

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
28.895	HSG A	1S, 2S
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.000	Other	
28.895		TOTAL AREA

Type III 24-hr 2yr Rainfall=2.76" Printed 8/5/2024

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 4

Time span=1.00-36.00 hrs, dt=0.05 hrs, 701 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: AREA FLOWING WEST TO Runoff Area=632,011 sf 0.78% Impervious Runoff Depth=0.00" Flow Length=1,950' Tc=42.0 min CN=31 Runoff=0.00 cfs 0.000 af

Subcatchment 2S: AREA FLOWING SOUTH TO Runoff Area=626,641 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=1,365' Tc=35.2 min CN=32 Runoff=0.00 cfs 0.000 af

Link POA1: POA1 AT KEARSARGE MTN RD Inflow=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af

Link POA2: AT WETLAND

Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Total Runoff Area = 28.895 ac Runoff Volume = 0.000 af Average Runoff Depth = 0.00" 99.61% Pervious = 28.782 ac 0.39% Impervious = 0.113 ac Prepared by HP

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 5

Summary for Subcatchment 1S: AREA FLOWING WEST TO EX. ROADWAY

Runoff 1.00 hrs, Volume= 0.000 af, Depth= 0.00" 0.00 cfs @

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 2yr Rainfall=2.76"

	Ar	rea (sf)	CN [Description					
*		2,004	98 E	98 Existing roadway, HSG A					
		35,850	39 >	⊳75% Ğras	s cover, Go	ood, HSG A			
	1	26,890	30 \	Woods, Go	od, HSG A				
		1,650	98 F	Paved park	ing, HSG A				
		1,271	98 F	Roofs, HSC	àΑ				
		4,450				ood, HSG A			
	4	59,896	30 \	Noods, Go	od, HSG A				
	6	32,011	31 \	Weighted A	verage				
	6	27,086	Ś	99.22% Per	vious Area				
		4,925	4,925 0.78% Impervious Area						
	_								
	Tc	Length	Slope	•	Capacity	Description			
(ı	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	23.1	100	0.0200	0.07		Sheet Flow, WOODS			
						Woods: Light underbrush n= 0.400 P2= 2.76"			
	14.3	900	0.0440	1.05		Shallow Concentrated Flow, WOODS			
						Woodland Kv= 5.0 fps			
	4.6	950	0.0500	3.45	38.85	Trap/Vee/Rect Channel Flow,			
						Bot.W=15.00' D=0.50' Z= 15.0 '/' Top.W=30.00'			
						n= 0.050			
	42.0	1,950	Total						

1,950 Total

Summary for Subcatchment 2S: AREA FLOWING SOUTH TO WETLAND

Runoff 1.00 hrs, Volume= 0.000 af, Depth= 0.00" 0.00 cfs @

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 2yr Rainfall=2.76"

Area (sf)	CN	Description
19,384	39	>75% Grass cover, Good, HSG A
131,943	30	Woods, Good, HSG A
85,506	39	>75% Grass cover, Good, HSG A
161,412	30	Woods, Good, HSG A
228,396	30	Woods, Good, HSG A
626,641	32	Weighted Average
626,641		100.00% Pervious Area

Type III 24-hr 2yr Rainfall=2.76" Printed 8/5/2024

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 6

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.3	100	0.0800	0.13		Sheet Flow,
٥.5		0.4000	4.50		Woods: Light underbrush n= 0.400 P2= 2.76"
9.5	900	0.1000	1.58		Shallow Concentrated Flow,
40.4	005	0.0040	0.40	40.00	Woodland Kv= 5.0 fps
12.4	365	0.0010	0.49	10.99	Trap/Vee/Rect Channel Flow,
					Bot.W=30.00' D=0.50' Z= 30.0 '/' Top.W=60.00'
					n= 0.050
 35.2	1,365	Total			

Summary for Link POA1: POA1 AT KEARSARGE MTN RD

Inflow Area = 14.509 ac, 0.78% Impervious, Inflow Depth = 0.00" for 2yr event

Inflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Summary for Link POA2: AT WETLAND

Inflow Area = 14.386 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2yr event

Inflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Prepared by HP

Type III 24-hr 10YR Rainfall=4.02" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 7

Time span=1.00-36.00 hrs, dt=0.05 hrs, 701 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: AREA FLOWING WEST TO Runoff Area=632,011 sf 0.78% Impervious Runoff Depth=0.00" Flow Length=1,950' Tc=42.0 min CN=31 Runoff=0.00 cfs 0.000 af

Subcatchment 2S: AREA FLOWING SOUTH TO Runoff Area=626,641 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=1,365' Tc=35.2 min CN=32 Runoff=0.00 cfs 0.000 af

Link POA1: POA1 AT KEARSARGE MTN RD Inflow=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af

Link POA2: AT WETLAND

Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Total Runoff Area = 28.895 ac Runoff Volume = 0.000 af Average Runoff Depth = 0.00" 99.61% Pervious = 28.782 ac 0.39% Impervious = 0.113 ac Prepared by HP

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 8

Summary for Subcatchment 1S: AREA FLOWING WEST TO EX. ROADWAY

Runoff = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.02"

_	Α	rea (sf)	CN [Description		
*		2,004	98 E	Existing roa	dway, HSG	S A
		35,850	39 >	>75% Gras	s cover, Go	ood, HSG A
	1	26,890	30 \	Noods, Go	od, HSG A	
		1,650	98 F	Paved park	ing, HSG A	
		1,271	98 F	Roofs, HSG	àΑ	
		4,450				ood, HSG A
_	4	59,896	30 \	Noods, Go	od, HSG A	
	6	32,011	31 \	Neighted A	verage	
	6	27,086	(99.22% Per	vious Area	
		4,925	().78% Impe	ervious Area	a
	Tc	Length	Slope	•	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	23.1	100	0.0200	0.07		Sheet Flow, WOODS
						Woods: Light underbrush n= 0.400 P2= 2.76"
			1.05		Shallow Concentrated Flow, WOODS	
					Woodland Kv= 5.0 fps	
	4.6 950 0.0500 3.45 3		38.85	Trap/Vee/Rect Channel Flow,		
						Bot.W=15.00' D=0.50' Z= 15.0 '/' Top.W=30.00'
_						n= 0.050
	42 N	1 050	Total			

42.0 1,950 Total

Summary for Subcatchment 2S: AREA FLOWING SOUTH TO WETLAND

Runoff = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.02"

Area (s	f) CN	Description
19,38	39	>75% Grass cover, Good, HSG A
131,94	3 30	Woods, Good, HSG A
85,50	6 39	>75% Grass cover, Good, HSG A
161,41	2 30	Woods, Good, HSG A
228,39	6 30	Woods, Good, HSG A
626,64	1 32	Weighted Average
626,64	-1	100.00% Pervious Area

KEARSARGE MTN RD WARNER PRE 8-04-24

Type III 24-hr 10YR Rainfall=4.02" Printed 8/5/2024

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 9

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	13.3	100	0.0800	0.13		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 2.76"
	9.5	900	0.1000	1.58		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
	12.4	365	0.0010	0.49	10.99	Trap/Vee/Rect Channel Flow, Bot.W=30.00' D=0.50' Z= 30.0 '/' Top.W=60.00' n= 0.050
-	25.0	1 OCE	Tatal			

35.2 1,365 Total

Summary for Link POA1: POA1 AT KEARSARGE MTN RD

Inflow Area = 14.509 ac, 0.78% Impervious, Inflow Depth = 0.00" for 10YR event

Inflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Summary for Link POA2: AT WETLAND

Inflow Area = 14.386 ac, 0.00% Impervious, Inflow Depth = 0.00" for 10YR event

Inflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

KEARSARGE MTN RD WARNER PRE 8-04-24

Prepared by HP

Type III 24-hr 25YR Rainfall=4.98" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 10

Time span=1.00-36.00 hrs, dt=0.05 hrs, 701 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: AREA FLOWING WEST TO Runoff Area=632,011 sf 0.78% Impervious Runoff Depth=0.01" Flow Length=1,950' Tc=42.0 min CN=31 Runoff=0.03 cfs 0.015 af

Subcatchment 2S: AREA FLOWING SOUTH TO Runoff Area=626,641 sf 0.00% Impervious Runoff Depth=0.02" Flow Length=1,365' Tc=35.2 min CN=32 Runoff=0.04 cfs 0.029 af

Link POA1: POA1 AT KEARSARGE MTN RD Inflow=0.03 cfs 0.015 af Primary=0.03 cfs 0.015 af

Link POA2: AT WETLAND Inflow=0.04 cfs 0.029 af Primary=0.04 cfs 0.029 af

Total Runoff Area = 28.895 ac Runoff Volume = 0.044 af Average Runoff Depth = 0.02" 99.61% Pervious = 28.782 ac 0.39% Impervious = 0.113 ac Prepared by HP

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 11

Summary for Subcatchment 1S: AREA FLOWING WEST TO EX. ROADWAY

Runoff = 0.03 cfs @ 23.10 hrs, Volume= 0.015 af, Depth= 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=4.98"

_	Α	rea (sf)	CN [Description		
*		2,004	98 E	Existing roa	dway, HSG	S A
		35,850	39 >	>75% Gras	s cover, Go	ood, HSG A
	1	26,890	30 \	Noods, Go	od, HSG A	
		1,650	98 F	Paved park	ing, HSG A	
		1,271	98 F	Roofs, HSG	àΑ	
		4,450				ood, HSG A
_	4	59,896	30 \	Noods, Go	od, HSG A	
	6	32,011	31 \	Neighted A	verage	
	6	27,086	(99.22% Per	vious Area	
		4,925	().78% Impe	ervious Area	a
	Tc	Length	Slope	•	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	23.1	100	0.0200	0.07		Sheet Flow, WOODS
						Woods: Light underbrush n= 0.400 P2= 2.76"
			1.05		Shallow Concentrated Flow, WOODS	
					Woodland Kv= 5.0 fps	
	4.6 950 0.0500 3.45 3		38.85	Trap/Vee/Rect Channel Flow,		
						Bot.W=15.00' D=0.50' Z= 15.0 '/' Top.W=30.00'
_						n= 0.050
	42 N	1 050	Total			

42.0 1,950 Total

Summary for Subcatchment 2S: AREA FLOWING SOUTH TO WETLAND

Runoff = 0.04 cfs @ 21.69 hrs, Volume= 0.029 af, Depth= 0.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=4.98"

Area (sf)	CN	Description
19,384	39	>75% Grass cover, Good, HSG A
131,943	30	Woods, Good, HSG A
85,506	39	>75% Grass cover, Good, HSG A
161,412	30	Woods, Good, HSG A
228,396	30	Woods, Good, HSG A
626,641	32	Weighted Average
626,641		100.00% Pervious Area

KEARSARGE MTN RD WARNER PRE 8-04-24

Type III 24-hr 25YR Rainfall=4.98" Printed 8/5/2024

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 12

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	13.3	100	0.0800	0.13		Sheet Flow,
	9.5	900	0.1000	1.58		Woods: Light underbrush n= 0.400 P2= 2.76" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
	12.4	365	0.0010	0.49	10.99	Trap/Vee/Rect Channel Flow, Bot.W=30.00' D=0.50' Z= 30.0 '/' Top.W=60.00'
-	25.2	1 265	Total			n= 0.050

35.2 1,365 Total

Summary for Link POA1: POA1 AT KEARSARGE MTN RD

Inflow Area = 14.509 ac, 0.78% Impervious, Inflow Depth = 0.01" for 25YR event

Inflow = 0.03 cfs @ 23.10 hrs, Volume= 0.015 af

Primary = 0.03 cfs @ 23.10 hrs, Volume= 0.015 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Summary for Link POA2: AT WETLAND

Inflow Area = 14.386 ac, 0.00% Impervious, Inflow Depth = 0.02" for 25YR event

Inflow = 0.04 cfs @ 21.69 hrs, Volume= 0.029 af

Primary = 0.04 cfs @ 21.69 hrs, Volume= 0.029 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

KEARSARGE MTN RD WARNER PRE 8-04-24

Prepared by HP

Type III 24-hr 50YR Rainfall=5.87" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 13

Time span=1.00-36.00 hrs, dt=0.05 hrs, 701 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: AREA FLOWING WEST TO Runoff Area=632,011 sf 0.78% Impervious Runoff Depth=0.08" Flow Length=1,950' Tc=42.0 min CN=31 Runoff=0.16 cfs 0.103 af

Subcatchment 2S: AREA FLOWING SOUTH TO Runoff Area=626,641 sf 0.00% Impervious Runoff Depth=0.11" Flow Length=1,365' Tc=35.2 min CN=32 Runoff=0.22 cfs 0.138 af

Link POA1: POA1 AT KEARSARGE MTN RDInflow=0.16 cfs 0.103 af Primary=0.16 cfs 0.103 af

Link POA2: AT WETLAND Inflow=0.22 cfs 0.138 af Primary=0.22 cfs 0.138 af

Total Runoff Area = 28.895 ac Runoff Volume = 0.240 af Average Runoff Depth = 0.10" 99.61% Pervious = 28.782 ac 0.39% Impervious = 0.113 ac Prepared by HP

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 14

Summary for Subcatchment 1S: AREA FLOWING WEST TO EX. ROADWAY

Runoff = 0.16 cfs @ 15.88 hrs, Volume= 0.103 af, Depth= 0.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 50YR Rainfall=5.87"

	Α	rea (sf)	CN E	escription		
*		2,004	98 E	xisting roa	dway, HSG	G A
		35,850	39 >	75% Gras	s cover, Go	ood, HSG A
	1	26,890			od, HSG A	
		1,650			ing, HSG A	
		1,271		Roofs, HSG	àΑ	
		4,450				ood, HSG A
_	4	59,896	<u>30 V</u>	Voods, Go	od, HSG A	
		32,011		Veighted A		
	6	27,086	_		vious Area	
		4,925	0	.78% Impe	ervious Area	a
	T					Description
	Tc (min)	Length	Slope	Velocity		Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Chart Flow WOODS
	23.1	100	0.0200	0.07		Sheet Flow, WOODS
	142	000	0.0440	1 05		Woods: Light underbrush n= 0.400 P2= 2.76"
	14.3 900		0.0440	1.05		Shallow Concentrated Flow, WOODS
			0.0500	2 15	20 05	Woodland Kv= 5.0 fps Trap/Vee/Rect Channel Flow,
	4.6 950 0.0500 3.45 38.85				30.03	Bot.W=15.00' D=0.50' Z= 15.0 '/' Top.W=30.00'
						n= 0.050
_	42 N	1 950	Total			11- 0.000

42.0 1,950 Total

Summary for Subcatchment 2S: AREA FLOWING SOUTH TO WETLAND

Runoff = 0.22 cfs @ 15.44 hrs, Volume= 0.138 af, Depth= 0.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 50YR Rainfall=5.87"

Area (sf)	CN	Description
19,384	39	>75% Grass cover, Good, HSG A
131,943	30	Woods, Good, HSG A
85,506	39	>75% Grass cover, Good, HSG A
161,412	30	Woods, Good, HSG A
228,396	30	Woods, Good, HSG A
626,641	32	Weighted Average
626,641		100.00% Pervious Area

KEARSARGE MTN RD WARNER PRE 8-04-24

Type III 24-hr 50YR Rainfall=5.87" Printed 8/5/2024

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 15

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.3	100	0.0800	0.13		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 2.76"
9.5	900	0.1000	1.58		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
12.4	365	0.0010	0.49	10.99	Trap/Vee/Rect Channel Flow,
					Bot.W=30.00' D=0.50' Z= 30.0 '/' Top.W=60.00'
					n= 0.050
35.2	1 365	Total			

1,365 Total

Summary for Link POA1: POA1 AT KEARSARGE MTN RD

Inflow Area = 14.509 ac, 0.78% Impervious, Inflow Depth = 0.08" for 50YR event

Inflow 0.16 cfs @ 15.88 hrs, Volume= 0.103 af

0.16 cfs @ 15.88 hrs, Volume= 0.103 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Summary for Link POA2: AT WETLAND

14.386 ac, 0.00% Impervious, Inflow Depth = 0.11" for 50YR event Inflow Area =

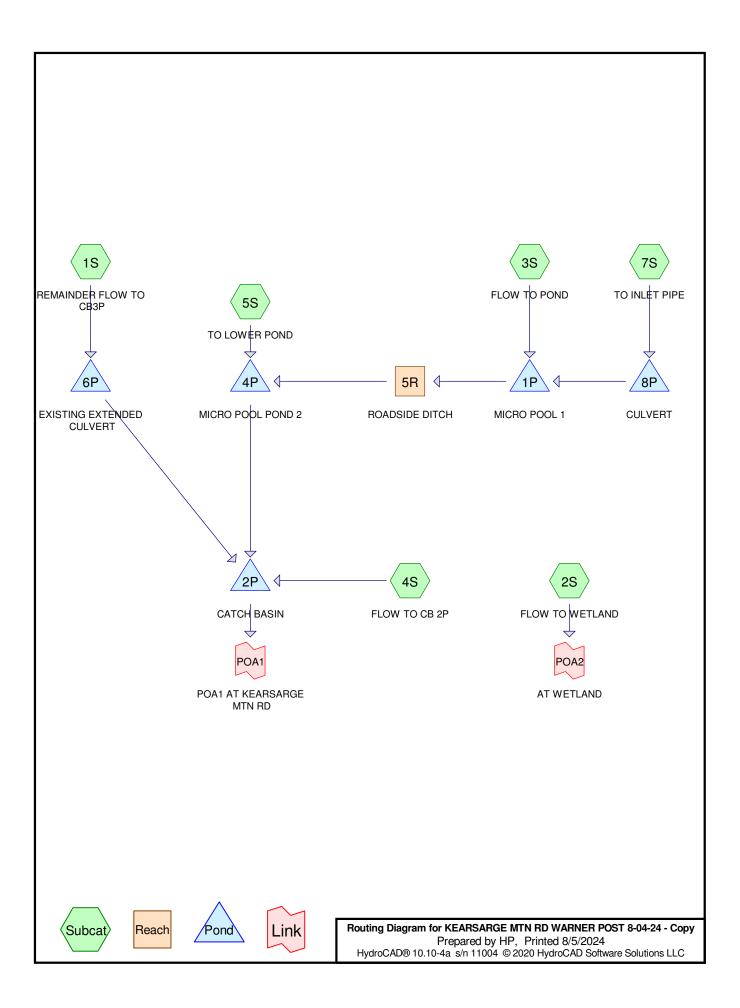
Inflow 0.22 cfs @ 15.44 hrs, Volume= 0.138 af

0.22 cfs @ 15.44 hrs, Volume= 0.138 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

STORMWATER MANAGEMENT REPORT Residential Subdivision Kearsarge MT. Road Warner NH

POST DEVELOPMENT DRAINAGE CALCULATIONS



Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Printed 8/5/2024 Page 2

Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
5.028	39	>75% Grass cover, Good, HSG A (1S, 2S, 3S, 4S, 5S, 7S)
0.046	98	Existing roadway, HSG A (1S)
0.467	98	Paved parking, HSG A (1S, 2S, 3S, 5S, 7S)
0.162	98	Paved roadway, HSG A (3S, 4S)
0.075	98	Roofs, HSG A (1S, 3S)
0.046	98	Unconnected roofs, HSG A (2S)
23.070	30	Woods, Good, HSG A (1S, 2S, 3S, 4S)
28.894	33	TOTAL AREA

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Printed 8/5/2024 Page 3

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
28.894	HSG A	1S, 2S, 3S, 4S, 5S, 7S
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.000	Other	
28.894		TOTAL AREA

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC Type III 24-hr 2yr Rainfall=2.76" Printed 8/5/2024

Page 4

Time span=1.00-36.00 hrs, dt=0.05 hrs, 701 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Pond 1P: MICRO POOL 1 Peak Elev=498.96' Storage=654 cf Inflow=0.16 cfs 0.015 af

Outflow=0.00 cfs 0.000 af

Subcatchment 1S: REMAINDER FLOW TO Runoff Area=570,080 sf 0.86% Impervious Runoff Depth=0.00"

Flow Length=1,950' Tc=49.4 min CN=31 Runoff=0.00 cfs 0.000 af

Pond 2P: CATCH BASIN Peak Elev=484.00' Storage=0.000 af Inflow=0.00 cfs 0.000 af

Discarded=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af

Subcatchment 2S: FLOW TO WETLAND Runoff Area=543,564 sf 0.63% Impervious Runoff Depth=0.00"

Flow Length=1,365' Tc=35.2 min CN=32 Runoff=0.00 cfs 0.000 af

Subcatchment 3S: FLOW TO POND Runoff Area=108,551 sf 10.79% Impervious Runoff Depth=0.00"

Flow Length=627' Tc=17.0 min CN=42 Runoff=0.00 cfs 0.000 af

Pond 4P: MICRO POOL POND 2 Peak Elev=485.89' Storage=493 cf Inflow=0.10 cfs 0.011 af

Outflow=0.00 cfs 0.000 af

Subcatchment 4S: FLOW TO CB 2P Runoff Area=7,292 sf 13.33% Impervious Runoff Depth=0.01"

Tc=6.0 min CN=45 Runoff=0.00 cfs 0.000 af

Reach 5R: ROADSIDE DITCH

Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af

n=0.050 L=180.0' S=0.0467'/' Capacity=11.71 cfs Outflow=0.00 cfs 0.000 af

Subcatchment 5S: TO LOWER POND Runoff Area=14,751 sf 43.31% Impervious Runoff Depth=0.40"

Tc=6.0 min CN=65 Runoff=0.10 cfs 0.011 af

Pond 6P: EXISTING EXTENDED CULVERT Peak Elev=487.73' Storage=0 cf Inflow=0.00 cfs 0.000 af

15.0" Round Culvert n=0.013 L=62.0' S=0.0553 '/' Outflow=0.00 cfs 0.000 af

Subcatchment 7S: TO INLET PIPE Runoff Area=14,399 sf 50.23% Impervious Runoff Depth=0.55"

Tc=6.0 min CN=69 Runoff=0.17 cfs 0.015 af

Pond 8P: CULVERT Peak Elev=499.62' Storage=10 cf Inflow=0.17 cfs 0.015 af

15.0" Round Culvert n=0.013 L=80.0' S=0.0050 '/' Outflow=0.16 cfs 0.015 af

Link POA1: POA1 AT KEARSARGE MTN RD Inflow=0.00 cfs 0.000 af

Primary=0.00 cfs 0.000 af

Link POA2: AT WETLAND Inflow=0.00 cfs 0.000 af

Primary=0.00 cfs 0.000 af

Total Runoff Area = 28.894 ac Runoff Volume = 0.026 af Average Runoff Depth = 0.01" 97.24% Pervious = 28.098 ac 2.76% Impervious = 0.796 ac

Type III 24-hr 2yr Rainfall=2.76" Printed 8/5/2024

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 5

Summary for Pond 1P: MICRO POOL 1

Inflow Area = 2.823 ac, 15.41% Impervious, Inflow Depth = 0.06" for 2yr event

Inflow = 0.16 cfs @ 12.13 hrs, Volume= 0.015 af

Outflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 498.96' @ 36.00 hrs Surf.Area= 569 sf Storage= 654 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Inve	ert Avail.Storage		Storage Description				
#1	497.0	0'	13,398 cf	Custom Stage Data (Irregular) Listed below (Recalc)				
Elevation	an .	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area		
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)		
497.0	00	150	48.0	0	0	150		
498.0	00	325	66.0	232	232	323		
499.9	90	875	102.0	1,098	1,330	830		
500.0	00	2,895	237.0	179	1,508	4,472		
502.0	00	4,310	276.0	7,158	8,667	6,145		
503.0	00	5,165	294.0	4,731	13,398	7,009		
Device	Routing	In	vert Outl	et Devices				
#1	Primary	498.50' 15.0		" Round Culvert L= 37.0' CPP, square edge headwall, Ke= 0.500				
			Inlet	/ Outlet Invert= 49	98.50' / 498.00' S:	= 0.0135 '/' Cc= 0.9	900	
				.013, Flow Area=	1.23 sf			
#2	Device 1	500	.00' 1.0"	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads				
#3	Device 1	502	.00' 24.0	'' x 24.0'' Horiz. Oı	rifice/Grate $C=0$	0.600		
Limited to weir flow at low heads								

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=497.00' TW=498.00' (Dynamic Tailwater)

1=Culvert (Controls 0.00 cfs)

2=Orifice/Grate (Controls 0.00 cfs)

U-3=Orifice/Grate (Controls 0.00 cfs)

Summary for Subcatchment 1S: REMAINDER FLOW TO CB3P

Runoff = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 2yr Rainfall=2.76"

Type III 24-hr 2yr Rainfall=2.76" Printed 8/5/2024

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 6

	Aı	rea (sf)	CN	Description								
*		2,004	98	Existing roadway, HSG A								
		35,850	39	>75% Gras	>75% Grass cover, Good, HSG A							
		64,959	30	Woods, Go	od, HSG A							
		1,650	98	Paved park	ing, HSG A							
		1,271	98	Roofs, HSG	àΑ							
		4,450		>75% Gras		ood, HSG A						
	4	59,896	30	Woods, Go	od, HSG A							
	5	70,080	31	Weighted A	verage							
	5	65,155		99.14% Per	vious Area							
		4,925		0.86% Impe	ervious Area	a						
	Тс	Length	Slope	•	Capacity	Description						
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)							
	30.5	100	0.0100	0.05		Sheet Flow, WOODS						
						Woods: Light underbrush n= 0.400 P2= 2.76"						
	14.3	900	0.0440	1.05		Shallow Concentrated Flow, WOODS						
						Woodland Kv= 5.0 fps						
	4.6	950	0.0500	3.45	38.85	•						
						Bot.W=15.00' D=0.50' Z= 15.0 '/' Top.W=30.00'						
						n= 0.050						
	49.4	1,950	Total									

Summary for Pond 2P: CATCH BASIN

Inflow Area =	16.416 ac,	4.37% Impervious, Inflow	Depth = 0.00"	for 2yr event
Inflow =	0.00 cfs @	22.22 hrs, Volume=	0.000 af	
Outflow =	0.00 cfs @	22.22 hrs, Volume=	0.000 af, Atte	en= 0%, Lag= 0.0 min
Discarded =	0.00 cfs @	22.22 hrs, Volume=	0.000 af	
Primary =	0.00 cfs @	1.00 hrs, Volume=	0.000 af	

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 484.00' @ 1.00 hrs Surf.Area= 0.010 ac Storage= 0.000 af Flood Elev= 488.00' Surf.Area= 0.010 ac Storage= 0.019 af

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 0.0 min (1,241.2 - 1,241.2)

Volume	Invert	Avail.Storage	Storage Description				
#1	484.00'	0.017 af	6.00'W x 75.00'L x 4.00'H Prismatoid				
#2	484.25'	0.002 af	0.041 af Overall x 40.0% Voids 15.0" Round Pipe Storage L= 75.0'				
		0.019 af Total Available Storage					
Device	Routing	Invert Ou	utlet Devices				
#1	Primary	484.30' 15	.0" Round Culvert L= 50.0' CPP, square edge headwall, Ke= 0.500				
			et / Outlet Invert= 484.30' / 483.80' S= 0.0100 '/' Cc= 0.900				
			0.010 PVC, smooth interior, Flow Area= 1.23 sf				
#2	Discarded	484.00' 3.0	000 in/hr Exfiltration over Surface area				

Type III 24-hr 2yr Rainfall=2.76" Printed 8/5/2024

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 7

Discarded OutFlow Max=0.00 cfs @ 22.22 hrs HW=484.00' (Free Discharge) **2=Exfiltration** (Passes 0.00 cfs of 0.03 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=484.00' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Controls 0.00 cfs)

Summary for Subcatchment 2S: FLOW TO WETLAND

Runoff = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 2yr Rainfall=2.76"

Aı	rea (sf)	CN I	Description						
	2,004	98 l	98 Unconnected roofs, HSG A						
	1,076	98 I	Paved park	ing, HSG A					
	20,000				ood, HSG A				
	45,170		,	od, HSG A					
	85,506				ood, HSG A				
1	61,412			od, HSG A					
_	370			ing, HSG A	L Company of the Comp				
	28,026			od, HSG A					
	43,564		Weighted A						
5	40,114		99.37% Pervious Area						
	3,450		0.63% Impervious Area						
	2,004	,	58.09% Und	connected					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description				
13.3	100	0.0800	0.13	(013)	Sheet Flow,				
10.0	100	0.0000	0.15		Woods: Light underbrush n= 0.400 P2= 2.76"				
9.5	900	0.1000	1.58		Shallow Concentrated Flow,				
0.0	000	0.1000	1.00		Woodland Kv= 5.0 fps				
12.4	365	0.0010	0.49	10.99	Trap/Vee/Rect Channel Flow,				
-			- 10	- 30	Bot.W=30.00' D=0.50' Z= 30.0 '/' Top.W=60.00'				
					n= 0.050				
35.2	1,365	Total							

Summary for Subcatchment 3S: FLOW TO POND

Runoff = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 2yr Rainfall=2.76"

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 8

	Aı	rea (sf)	CN	Description								
		2,004	98	Roofs, HSG	àΑ							
*		6,098	98	Paved roadway, HSG A								
		3,615	98	Paved park	ing, HSG A							
		53,013	39	>75% Gras	s cover, Go	od, HSG A						
		43,821		Woods, Go								
	1	08,551	42	Weighted A	verage							
		96,834		89.21% Per								
		11,717		10.79% lmp	pervious Are	ea						
	Tc	Length	Slope	e Velocity	Capacity	Description						
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)							
	14.0	100	0.0700	0.12		Sheet Flow,						
						Woods: Light underbrush n= 0.400 P2= 2.76"						
	0.7	90	0.2000	2.24		Shallow Concentrated Flow,						
						Woodland Kv= 5.0 fps						
	0.1	25	0.3300	4.02		Shallow Concentrated Flow,						
						Short Grass Pasture Kv= 7.0 fps						
	2.2	412	0.0300	3.13	9.39	Trap/Vee/Rect Channel Flow,						
						Bot.W=0.00' D=1.00' Z= 3.0 '/' Top.W=6.00'						
						n= 0.050						
	17.0	627	Total									

Summary for Pond 4P: MICRO POOL POND 2

Inflow Area = 3.161 ac, 18.40% Impervious, Inflow Depth = 0.04" for 2yr event

Inflow = 0.10 cfs @ 12.12 hrs, Volume= 0.011 af

Outflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 485.89' @ 24.40 hrs Surf.Area= 440 sf Storage= 493 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage	e Description	
#1	484.00'	4,477 cf	Custon	n Stage Data (Pri	smatic) Listed below (Recalc)
Elevation	Surf.A		c.Store	Cum.Store	
(feet)	(SC	լ-ft) (cub	ic-feet)	(cubic-feet)	
484.00		80	0	0	
486.00	4	460	540	540	
486.90	7	750	544	1,084	
487.00	1,0	051	90	1,175	
488.00	1,6	633	1,342	2,517	
489.00	2,2	288	1,961	4,477	

Type III 24-hr 2yr Rainfall=2.76" Printed 8/5/2024

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 9

Device	Routing	Invert	Outlet Devices
#1	Primary	484.80'	15.0" Round Culvert L= 46.7' Ke= 0.500
	-		Inlet / Outlet Invert= 484.80' / 484.40' S= 0.0086 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Device 1	487.00'	1.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	488.25'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=484.00' TW=484.00' (Dynamic Tailwater)

1=Culvert (Controls 0.00 cfs)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Controls 0.00 cfs)

Summary for Subcatchment 4S: FLOW TO CB 2P

Runoff = 0.00 cfs @ 22.22 hrs, Volume= 0.000 af, Depth= 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 2yr Rainfall=2.76"

	Area (sf)	CN	Description									
*	972	98	Paved road	Paved roadway, HSG A								
	4,670	39	>75% Gras	s cover, Go	Good, HSG A							
	1,650	30	Woods, Go	Woods, Good, HSG A								
	7,292	45	Weighted A	Weighted Average								
	6,320		86.67% Per	rvious Area	a							
	972		13.33% lmp	pervious Ar	rea							
_												
To	- 3-	Slop	,	Capacity	· · · · · · · · · · · · · · · · · · ·							
(min)) (feet)	(ft/f	t) (ft/sec) (cfs)									
6.0)				Direct Entry,							

Summary for Reach 5R: ROADSIDE DITCH

Inflow Area = 2.823 ac. 15.41% Impervious. Inflow Depth = 0.00" for 2vr event

Inflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 1.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 11.71 cfs

0.00' x 1.00' deep channel, n= 0.050

Side Slope Z-value= 3.0 '/' Top Width= 6.00'

Length= 180.0' Slope= 0.0467 '/'

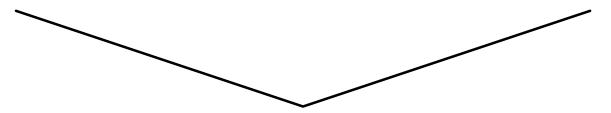
Inlet Invert= 498.00', Outlet Invert= 489.60'

Prepared by HP

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Type III 24-hr 2yr Rainfall=2.76" Printed 8/5/2024

Page 10



Summary for Subcatchment 5S: TO LOWER POND

Runoff = 0.10 cfs @ 12.12 hrs, Volume= 0.011 af, Depth= 0.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 2yr Rainfall=2.76"

Area (sf)	CN	Description								
8,362	39	>75% Gras	Good, HSG A							
6,389	98	Paved park	Paved parking, HSG A							
14,751	65	Weighted A	verage							
8,362		56.69% Pervious Area								
6,389		43.31% lmp	pervious Ar	rea						
Tc Lengt		,	Capacity	•						
(min) (feet	t) (ft/	t) (ft/sec) (cfs)								
6.0				Direct Entry,						

Summary for Pond 6P: EXISTING EXTENDED CULVERT

Inflow Area = 13.087 ac, 0.86% Impervious, Inflow Depth = 0.00" for 2yr event

Inflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Peak Elev= 487.73' @ 1.00 hrs Surf.Area= 12 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no inflow)

Volume	Inve	ert Ava	il.Storage	Storage	e Description	
#1	487.7	'3'	1,003 cf	Custon	n Stage Data (Prisn	natic) Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)	_	Store -feet)	Cum.Store (cubic-feet)	
487.7	'3	12		0	0	
488.0	0	50		8	8	
490.0	0	945		995	1,003	
Device	Routing	Ir	vert Outle	et Device	es	
#1	Primary	487	7.83' 15.0'	' Round	d Culvert L= 62.0'	Ke= 0.500

15.0" Round Culvert L= 62.0' Ke= 0.500 Inlet / Outlet Invert= 487.83' / 484.40' S= 0.0553 '/' Cc= 0.900

n= 0.013, Flow Area= 1.23 sf

Type III 24-hr 2yr Rainfall=2.76" Printed 8/5/2024

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 11

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=487.73' TW=484.00' (Dynamic Tailwater) **1=Culvert** (Controls 0.00 cfs)

Summary for Subcatchment 7S: TO INLET PIPE

Runoff = 0.17 cfs @ 12.11 hrs, Volume= 0.015 af, Depth= 0.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 2yr Rainfall=2.76"

Area	(sf) CN	Description	Description								
7,	233 98	Paved park	Paved parking, HSG A								
7,	166 39	>75% Gras	>75% Grass cover, Good, HSG A								
14,	399 69	Weighted A	Weighted Average								
7,	166	49.77% Per	49.77% Pervious Area								
7,	233	50.23% lmp	pervious Are	ea							
- .	01			.							
	ength Slo	,	Capacity	Description							
(min)	(feet) (ft	/ft) (ft/sec)	(cfs)								
6.0				Direct Entry,							

Summary for Pond 8P: CULVERT

Inflow Area = 0.331 ac, 50.23% Impervious, Inflow Depth = 0.55" for 2yr event

Inflow = 0.17 cfs @ 12.11 hrs, Volume= 0.015 af

Outflow = 0.16 cfs @ 12.13 hrs, Volume= 0.015 af, Atten= 3%, Lag= 0.9 min

Primary = 0.16 cfs @ 12.13 hrs, Volume= 0.015 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Peak Elev= 499.62' @ 12.13 hrs Surf.Area= 70 sf Storage= 10 cf

Plug-Flow detention time= 2.3 min calculated for 0.015 af (100% of inflow)

Center-of-Mass det. time= 2.3 min (894.5 - 892.2)

Volume	Inve	ert Avail.Storage		Storage	Description			
#1	499.4	10'	253 cf	Custom	Stage Data (Pris	matic) Listed below (Recalc)		
Elevatio (feet		Surf.Area (sq-ft)		c.Store c-feet)	Cum.Store (cubic-feet)			
499.4	0	25	0		0			
500.0	0	150		53	53			
501.0	0	250		200	253			
Device	Routing	Inv	ert Outl	let Device	S			
#1	Primary	499.4	99.40' 15.0" Round Culvert L= 80.0' Ke= 0.500 Inlet / Outlet Invert= 499.40' / 499.00' S= 0.0050 '/' Cc= 0.900					
Inlet / Outlet Invert= 499.40' / 499.00' S= 0.0050 '/' Cc= 0.900								

Inlet / Outlet Invert= 499.40' / 499.00' S= 0.0050 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=0.16 cfs @ 12.13 hrs HW=499.61' TW=497.38' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 0.16 cfs @ 1.72 fps)

Prepared by HP

Type III 24-hr 2yr Rainfall=2.76" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 12

Summary for Link POA1: POA1 AT KEARSARGE MTN RD

Inflow Area = 16.416 ac, 4.37% Impervious, Inflow Depth = 0.00" for 2yr event

Inflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Summary for Link POA2: AT WETLAND

Inflow Area = 12.479 ac, 0.63% Impervious, Inflow Depth = 0.00" for 2yr event

Inflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Prepared by HP

Type III 24-hr 10YR Rainfall=4.02" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 13

Time span=1.00-36.00 hrs, dt=0.05 hrs, 701 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Pond 1P: MICRO POOL 1 Peak Elev=500.22' Storage=2,167 cf Inflow=0.46 cfs 0.056 af

Outflow=0.01 cfs 0.014 af

Subcatchment 1S: REMAINDER FLOW TO Runoff Area=570,080 sf 0.86% Impervious Runoff Depth=0.00"

Flow Length=1,950' Tc=49.4 min CN=31 Runoff=0.00 cfs 0.000 af

Pond 2P: CATCH BASIN Peak Elev=484.00' Storage=0.000 af Inflow=0.01 cfs 0.015 af

Discarded=0.01 cfs 0.015 af Primary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.015 af

Subcatchment 2S: FLOW TO WETLAND Runoff Area=543,564 sf 0.63% Impervious Runoff Depth=0.00"

Flow Length=1,365' Tc=35.2 min CN=32 Runoff=0.00 cfs 0.000 af

Subcatchment 3S: FLOW TO POND Runoff Area=108,551 sf 10.79% Impervious Runoff Depth=0.11"

Flow Length=627' Tc=17.0 min CN=42 Runoff=0.04 cfs 0.022 af

Pond 4P: MICRO POOL POND 2 Peak Elev=487.20' Storage=1,391 cf Inflow=0.36 cfs 0.043 af

Outflow=0.01 cfs 0.012 af

Subcatchment 4S: FLOW TO CB 2P Runoff Area=7,292 sf 13.33% Impervious Runoff Depth=0.18"

Tc=6.0 min CN=45 Runoff=0.01 cfs 0.003 af

Reach 5R: ROADSIDE DITCH

Avg. Flow Depth=0.07' Max Vel=0.69 fps Inflow=0.01 cfs 0.014 af

n=0.050 L=180.0' S=0.0467'/' Capacity=11.71 cfs Outflow=0.01 cfs 0.014 af

Subcatchment 5S: TO LOWER POND Runoff Area=14,751 sf 43.31% Impervious Runoff Depth=1.04"

Tc=6.0 min CN=65 Runoff=0.36 cfs 0.029 af

Pond 6P: EXISTING EXTENDED CULVERT Peak Elev=487.73' Storage=0 cf Inflow=0.00 cfs 0.000 af

15.0" Round Culvert n=0.013 L=62.0' S=0.0553 '/' Outflow=0.00 cfs 0.000 af

Subcatchment 7S: TO INLET PIPE Runoff Area=14,399 sf 50.23% Impervious Runoff Depth=1.28"

Tc=6.0 min CN=69 Runoff=0.46 cfs 0.035 af

Pond 8P: CULVERT Peak Elev=500.22' Storage=88 cf Inflow=0.46 cfs 0.035 af

15.0" Round Culvert n=0.013 L=80.0' S=0.0050 '/' Outflow=0.46 cfs 0.034 af

Link POA1: POA1 AT KEARSARGE MTN RD Inflow=0.00 cfs 0.000 af

Primary=0.00 cfs 0.000 af

Link POA2: AT WETLAND Inflow=0.00 cfs 0.000 af

Primary=0.00 cfs 0.000 af

Total Runoff Area = 28.894 ac Runoff Volume = 0.089 af Average Runoff Depth = 0.04" 97.24% Pervious = 28.098 ac 2.76% Impervious = 0.796 ac

Prepared by HP

Type III 24-hr 10YR Rainfall=4.02" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 14

Summary for Pond 1P: MICRO POOL 1

Inflow Area = 2.823 ac, 15.41% Impervious, Inflow Depth > 0.24" for 10YR event

Inflow = 0.46 cfs @ 12.11 hrs, Volume= 0.056 af

Outflow = 0.01 cfs @ 24.18 hrs, Volume= 0.014 af, Atten= 98%, Lag= 723.9 min

Primary = 0.01 cfs @ 24.18 hrs, Volume= 0.014 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 500.22' @ 24.18 hrs Surf.Area= 3,038 sf Storage= 2,167 cf

Plug-Flow detention time= 856.8 min calculated for 0.014 af (25% of inflow)

Center-of-Mass det. time= 640.6 min (1,583.8 - 943.3)

Volume	Inv	ert Ava	il.Storage	Storage Description						
#1	497.0	00'	13,398 cf	Custom Stage D	ata (Irregular) List	ted below (Recalc)				
Elevatio		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)				
497.0	00	150	48.0	0	0	150				
498.0	00	325	66.0	232	232	323				
499.9	90	875	102.0	1,098	1,330	830				
500.0	00	2,895	237.0	179	1,508	4,472				
502.0	00	4,310	276.0	7,158	8,667	6,145				
503.0	00	5,165	294.0	4,731	13,398	7,009				
Device	Routing	lr	nvert Out	et Devices						
#1	Primary	498	3.50' 15.0	" Round Culvert	L= 37.0' CPP, s	quare edge headw	all, Ke= 0.500			
			Inle	t / Outlet Invert= 49	98.50' / 498.00' S	= 0.0135 '/' Cc= 0	.900			
			n= 0.013, Flow Area= 1.23 sf							
#2	2 Device 1 500.00' 1.0" Ve		Vert. Orifice/Grat	e C= 0.600 Lim	nited to weir flow at	low heads				
#3 Device 1 502.00' 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads										

Primary OutFlow Max=0.01 cfs @ 24.18 hrs HW=500.22' TW=498.07' (Dynamic Tailwater)

1=Culvert (Passes 0.01 cfs of 6.19 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.01 cfs @ 2.05 fps)

3=Orifice/Grate (Controls 0.00 cfs)

Summary for Subcatchment 1S: REMAINDER FLOW TO CB3P

Runoff = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.02"

KEARSARGE MTN RD WARNER POST 8-04-24 - CopyPrepared by HP

Type III 24-hr 10YR Rainfall=4.02" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 15

_	A	rea (sf)	CN I	Description						
*		2,004	98 I	98 Existing roadway, HSG A						
		35,850	39	>75% Gras	s cover, Go	ood, HSG A				
		64,959	30 \	Woods, Go	od, HSG A					
		1,650	98 I	Paved park	ing, HSG A					
		1,271	98 I	Roofs, HSG	àΑ					
		4,450	39 :	>75% Gras	s cover, Go	ood, HSG A				
_	4	59,896	30 \	Woods, Go	od, HSG A					
	5	70,080	31 \	Neighted A	verage					
	5	65,155	Ç	99.14% Per	vious Area					
		4,925	(0.86% lmpe	ervious Area	a				
	Tc	Length	Slope	•	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	30.5	100	0.0100	0.05		Sheet Flow, WOODS				
						Woods: Light underbrush n= 0.400 P2= 2.76"				
	14.3	900	0.0440	1.05		Shallow Concentrated Flow, WOODS				
						Woodland Kv= 5.0 fps				
	4.6	950	0.0500	3.45	38.85	Trap/Vee/Rect Channel Flow,				
						Bot.W=15.00' D=0.50' Z= 15.0'/' Top.W=30.00'				
_						n= 0.050				
	49.4	1,950	Total							

Summary for Pond 2P: CATCH BASIN

Inflow Area =	16.416 ac,	4.37% Impervious, Inflow I	Depth > 0.01" for 10YR event
Inflow =	0.01 cfs @	23.98 hrs, Volume=	0.015 af
Outflow =	0.01 cfs @	23.98 hrs, Volume=	0.015 af, Atten= 0%, Lag= 0.0 min
Discarded =	0.01 cfs @	23.98 hrs, Volume=	0.015 af
Primary =	0.00 cfs @	1.00 hrs, Volume=	0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 484.00' @ 1.00 hrs Surf.Area= 0.010 ac Storage= 0.000 af Flood Elev= 488.00' Surf.Area= 0.010 ac Storage= 0.019 af

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= (not calculated: outflow precedes inflow)

<u>Volume</u>	Invert	Avail.Storage	Storage Description
#1	484.00'	0.017 af	6.00'W x 75.00'L x 4.00'H Prismatoid
#2	484.25'	0.002 af	0.041 af Overall x 40.0% Voids 15.0" Round Pipe Storage L= 75.0'
		0.019 af	Total Available Storage
Device	Routing	Invert Ou	utlet Devices
#1	Primary	Inl	6.0" Round Culvert L= 50.0' CPP, square edge headwall, Ke= 0.500 et / Outlet Invert= 484.30' / 483.80' S= 0.0100 '/' Cc= 0.900
#2	Discarded		0.010 PVC, smooth interior, Flow Area= 1.23 sf Oo in/hr Exfiltration over Surface area

Prepared by HP

Type III 24-hr 10YR Rainfall=4.02" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 16

Discarded OutFlow Max=0.00 cfs @ 23.98 hrs HW=484.00' (Free Discharge) **2=Exfiltration** (Passes 0.00 cfs of 0.03 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=484.00' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Controls 0.00 cfs)

Summary for Subcatchment 2S: FLOW TO WETLAND

Runoff = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.02"

Ar	rea (sf)	CN [Description					
	2,004	98 l	Jnconnected roofs, HSG A					
	1,076	98 F	Paved park	ing, HSG A	L			
	20,000	39 >	>75% Gras	s cover, Go	ood, HSG A			
	45,170		,	od, HSG A				
	85,506				ood, HSG A			
1	61,412		,	od, HSG A				
	370		•	ing, HSG A				
	28,026			od, HSG A				
	43,564		Neighted A					
5	40,114			vious Area				
	3,450			ervious Are	a			
	2,004	Ę	58.09% Und	connected				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description			
13.3	100	0.0800		(0.0)	Sheet Flow,			
10.0	100	0.0000	0.10		Woods: Light underbrush n= 0.400 P2= 2.76"			
9.5	900	0.1000	1.58		Shallow Concentrated Flow,			
0.0		01.000			Woodland Kv= 5.0 fps			
12.4	365	0.0010	0.49	10.99	•			
					Bot.W=30.00' D=0.50' Z= 30.0 '/' Top.W=60.00'			
					n= 0.050			
35.2	1,365	Total						

Summary for Subcatchment 3S: FLOW TO POND

Runoff = 0.04 cfs @ 14.82 hrs, Volume= 0.022 af, Depth= 0.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.02"

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 17

	Α	rea (sf)	CN	Description						
		2,004	98	Roofs, HSG A						
*		6,098	98	Paved road	way, HSG	A				
		3,615			ing, HSG A					
		53,013				ood, HSG A				
		43,821			od, HSG A	•				
_	1	08,551	42	Weighted A	verage					
		96,834			vious Area					
		11,717		10.79% lmp	pervious Are	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	<u> </u>				
	14.0	100	0.0700	0.12		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 2.76"				
	0.7	90	0.2000	2.24		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	0.1	25	0.3300	4.02		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	2.2	412	0.0300 3.13 9.39		9.39	Trap/Vee/Rect Channel Flow,				
						Bot.W=0.00' D=1.00' Z= 3.0 '/' Top.W=6.00'				
_						n= 0.050				
	17.0	627	Total							

Summary for Pond 4P: MICRO POOL POND 2

Inflow Area = 3.161 ac, 18.40% Impervious, Inflow Depth > 0.16" for 10YR event

Inflow = 0.36 cfs @ 12.10 hrs, Volume= 0.043 af

Outflow = 0.01 cfs @ 26.60 hrs, Volume= 0.012 af, Atten= 97%, Lag= 869.5 min

Primary = 0.01 cfs @ 26.60 hrs, Volume= 0.012 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 487.20' @ 26.60 hrs Surf.Area= 1,165 sf Storage= 1,391 cf

Plug-Flow detention time= 968.8 min calculated for 0.012 af (28% of inflow)

Center-of-Mass det. time= 597.5 min (1,700.9 - 1,103.4)

Volume	Invert	Avail.Storage	Storage	e Description	
#1	484.00'	4,477 cf	Custon	n Stage Data (Pri	smatic) Listed below (Recalc)
Elevation	Surf.A		c.Store	Cum.Store	
(feet)	(SC	լ-ft) (cub	ic-feet)	(cubic-feet)	
484.00		80	0	0	
486.00	4	460	540	540	
486.90	7	750	544	1,084	
487.00	1,0	051	90	1,175	
488.00	1,6	633	1,342	2,517	
489.00	2,2	288	1,961	4,477	

Type III 24-hr 10YR Rainfall=4.02" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 18

Device	Routing	Invert	Outlet Devices
#1	Primary	484.80'	15.0" Round Culvert L= 46.7' Ke= 0.500
	-		Inlet / Outlet Invert= 484.80' / 484.40' S= 0.0086 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Device 1	487.00'	1.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	488.25'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=0.01 cfs @ 26.60 hrs HW=487.20' TW=484.00' (Dynamic Tailwater)

1=Culvert (Passes 0.01 cfs of 7.61 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 0.01 cfs @ 1.89 fps)

3=Orifice/Grate (Controls 0.00 cfs)

Prepared by HP

Summary for Subcatchment 4S: FLOW TO CB 2P

Runoff = 0.01 cfs @ 12.45 hrs, Volume= 0.003 af, Depth= 0.18"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.02"

A	rea (sf)	CN	Description							
*	972	98	Paved road	way, HSG	Α					
	4,670	39	>75% Gras	s cover, Go	ood, HSG A					
	1,650	30	Woods, Go	od, HSG A						
	7,292	45	Weighted A	Weighted Average						
	6,320		86.67% Per	vious Area						
	972		13.33% lmp	pervious Are	ea					
Tc (min)	Length (feet)	Slop (ft/f	•	Capacity (cfs)	Description					
6.0					Direct Entry,					

Summary for Reach 5R: ROADSIDE DITCH

Inflow Area = 2.823 ac. 15.41% Impervious, Inflow Depth > 0.06" for 10YR event

Inflow = 0.01 cfs @ 24.18 hrs, Volume= 0.014 af

Outflow = 0.01 cfs @ 24.23 hrs, Volume= 0.014 af, Atten= 0%, Lag= 3.3 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.69 fps, Min. Travel Time= 4.4 min Avg. Velocity = 0.63 fps, Avg. Travel Time= 4.7 min

Peak Storage= 3 cf @ 24.23 hrs

Average Depth at Peak Storage= 0.07', Surface Width= 0.44' Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 11.71 cfs

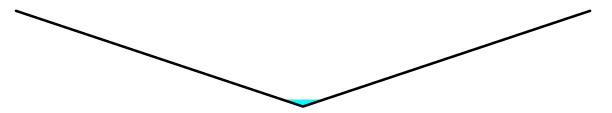
0.00' x 1.00' deep channel, n= 0.050 Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 180.0' Slope= 0.0467 '/' Inlet Invert= 498.00', Outlet Invert= 489.60'

Prepared by HP

Type III 24-hr 10YR Rainfall=4.02" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 19



Summary for Subcatchment 5S: TO LOWER POND

Runoff = 0.36 cfs @ 12.10 hrs, Volume= 0.029 af, Depth= 1.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.02"

Area (sf)	CN	Description	Description						
8,362	39	>75% Gras	>75% Grass cover, Good, HSG A						
6,389	98	Paved park	ing, HSG A	A					
14,751	65	Weighted A	Weighted Average						
8,362		56.69% Per	56.69% Pervious Area						
6,389		43.31% lmp	pervious Ar	rea					
Tc Lengt		,	Capacity	•					
(min) (feet	t) (ft/	ft) (ft/sec)	(cfs)						
6.0				Direct Entry,					

Summary for Pond 6P: EXISTING EXTENDED CULVERT

Inflow Area = 13.087 ac, 0.86% Impervious, Inflow Depth = 0.00" for 10YR event

Inflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Peak Elev= 487.73' @ 1.00 hrs Surf.Area= 12 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no inflow)

Volume	Inve	ert Ava	il.Storage	Storage	Description	
#1	487.7	' 3'	1,003 cf	Custom	n Stage Data (Prisn	natic) Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)	_	Store c-feet)	Cum.Store (cubic-feet)	
487.7	'3	12		0	0	
488.0	0	50		8	8	
490.0	0	945		995	1,003	
Device	Routing	Ir	nvert Outle	et Device	es	
#1	Primary	487	7.83' 15.0'	' Round	Culvert L= 62.0'	Ke= 0.500

15.0" Round Culvert L= 62.0' Ke= 0.500 Inlet / Outlet Invert= 487.83' / 484.40' S= 0.0553 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf

Prepared by HP

Type III 24-hr 10YR Rainfall=4.02" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 20

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=487.73' TW=484.00' (Dynamic Tailwater) **1=Culvert** (Controls 0.00 cfs)

Summary for Subcatchment 7S: TO INLET PIPE

Runoff = 0.46 cfs @ 12.10 hrs, Volume= 0.035 af, Depth= 1.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.02"

Ar	rea (sf)	CN	Description							
	7,233	98	Paved parking, HSG A							
	7,166	39	>75% Gras	s cover, Go	od, HSG A					
	14,399	69	Weighted A	verage						
	7,166		49.77% Per	vious Area						
	7,233		50.23% Imp	pervious Ar	ea					
т.	l	Olara		0	Danawintina					
Tc	Length	Slope	,	Capacity	Description					
<u>(min)</u>	(feet)	(ft/ft) (ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Pond 8P: CULVERT

Inflow Area = 0.331 ac, 50.23% Impervious, Inflow Depth = 1.28" for 10YR event

Inflow = 0.46 cfs @ 12.10 hrs, Volume= 0.035 af

Outflow = 0.46 cfs @ 12.11 hrs, Volume= 0.034 af, Atten= 1%, Lag= 0.7 min

Primary = 0.46 cfs @ 12.11 hrs, Volume= 0.034 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Peak Elev= 500.22' @ 24.14 hrs Surf.Area= 172 sf Storage= 88 cf

Plug-Flow detention time= 28.2 min calculated for 0.034 af (95% of inflow)

Center-of-Mass det. time= 5.5 min (868.7 - 863.2)

Volume	Invert	Avail.Sto	rage Sto	rage D	escription		
#1	499.40'	25	53 cf Cu	stom S	tage Data (Pri	smatic) Listed below (Recalc)	
Elevation (feet)		f.Area (sq-ft)	Inc.Sto	_	Cum.Store (cubic-feet)		
499.40		25		0	0		
500.00		150	5	53	53		
501.00		250	20	00	253		
Device R	outing	Invert	Outlet D	evices			
#1 P	#1 Primary 499.40' 15.0" Round Culvert L= 80.0' Ke= 0.500 Inlet / Outlet Invert= 499.40' / 499.00' S= 0.0050 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf						

Primary OutFlow Max=0.44 cfs @ 12.11 hrs HW=499.76' TW=498.14' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 0.44 cfs @ 2.29 fps)

Prepared by HP

Type III 24-hr 10YR Rainfall=4.02" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 21

Summary for Link POA1: POA1 AT KEARSARGE MTN RD

Inflow Area = 16.416 ac, 4.37% Impervious, Inflow Depth = 0.00" for 10YR event

Inflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Summary for Link POA2: AT WETLAND

Inflow Area = 12.479 ac, 0.63% Impervious, Inflow Depth = 0.00" for 10YR event

Inflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Prepared by HP

Type III 24-hr 25YR Rainfall=4.98" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 22

Time span=1.00-36.00 hrs, dt=0.05 hrs, 701 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Pond 1P: MICRO POOL 1 Peak Elev=500.82' Storage=4,112 cf Inflow=0.72 cfs 0.114 af

Outflow=0.02 cfs 0.039 af

Subcatchment 1S: REMAINDER FLOW TO Runoff Area=570,080 sf 0.86% Impervious Runoff Depth=0.01"

Flow Length=1,950' Tc=49.4 min CN=31 Runoff=0.03 cfs 0.013 af

Pond 2P: CATCH BASIN Peak Elev=484.36' Storage=0.002 af Inflow=0.05 cfs 0.056 af

Discarded=0.03 cfs 0.051 af Primary=0.02 cfs 0.005 af Outflow=0.05 cfs 0.056 af

Subcatchment 2S: FLOW TO WETLAND Runoff Area=543,564 sf 0.63% Impervious Runoff Depth=0.02"

Flow Length=1,365' Tc=35.2 min CN=32 Runoff=0.04 cfs 0.025 af

Subcatchment 3S: FLOW TO POND Runoff Area=108,551 sf 10.79% Impervious Runoff Depth=0.31"

Flow Length=627' Tc=17.0 min CN=42 Runoff=0.22 cfs 0.064 af

Pond 4P: MICRO POOL POND 2 Peak Elev=487.75' Storage=2,128 cf Inflow=0.61 cfs 0.085 af

Outflow=0.02 cfs 0.037 af

Subcatchment 4S: FLOW TO CB 2P Runoff Area=7,292 sf 13.33% Impervious Runoff Depth=0.44"

Tc=6.0 min CN=45 Runoff=0.03 cfs 0.006 af

Reach 5R: ROADSIDE DITCH

Avg. Flow Depth=0.10' Max Vel=0.83 fps Inflow=0.02 cfs 0.039 af

n=0.050 L=180.0' S=0.0467 '/' Capacity=11.71 cfs Outflow=0.02 cfs 0.038 af

Subcatchment 5S: TO LOWER POND Runoff Area=14,751 sf 43.31% Impervious Runoff Depth=1.64"

Tc=6.0 min CN=65 Runoff=0.61 cfs 0.046 af

Pond 6P: EXISTING EXTENDED CULVERT Peak Elev=487.90' Storage=4 cf Inflow=0.03 cfs 0.013 af

15.0" Round Culvert n=0.013 L=62.0' S=0.0553 '/' Outflow=0.03 cfs 0.013 af

Subcatchment 7S: TO INLET PIPE Runoff Area=14,399 sf 50.23% Impervious Runoff Depth=1.94"

Tc=6.0 min CN=69 Runoff=0.72 cfs 0.054 af

Pond 8P: CULVERT Peak Elev=500.82' Storage=209 cf Inflow=0.72 cfs 0.054 af

15.0" Round Culvert n=0.013 L=80.0' S=0.0050 '/' Outflow=0.72 cfs 0.050 af

Link POA1: POA1 AT KEARSARGE MTN RD Inflow=0.02 cfs 0.005 af

Primary=0.02 cfs 0.005 af

Link POA2: AT WETLAND Inflow=0.04 cfs 0.025 af

Primary=0.04 cfs 0.025 af

Total Runoff Area = 28.894 ac Runoff Volume = 0.208 af Average Runoff Depth = 0.09" 97.24% Pervious = 28.098 ac 2.76% Impervious = 0.796 ac

Prepared by HP

Type III 24-hr 25YR Rainfall=4.98" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 23

Summary for Pond 1P: MICRO POOL 1

Inflow Area = 2.823 ac, 15.41% Impervious, Inflow Depth > 0.48" for 25YR event

Inflow 0.72 cfs @ 12.11 hrs, Volume= 0.114 af

0.02 cfs @ 24.17 hrs, Volume= Outflow 0.039 af, Atten= 97%, Lag= 723.7 min

Primary 0.02 cfs @ 24.17 hrs, Volume= 0.039 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 500.82' @ 24.17 hrs Surf.Area= 3,443 sf Storage= 4,112 cf

Plug-Flow detention time= 760.3 min calculated for 0.039 af (34% of inflow)

Center-of-Mass det. time= 564.4 min (1,497.9 - 933.5)

Volume	Inv	ert Ava	il.Storage	e Storage Description					
#1 497.00' 13,398		13,398 cf	Custom Stage D	Custom Stage Data (Irregular) Listed below (Recalc)					
Elevatio (fee		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
497.0	0	150	48.0	0	0	150			
498.0	0	325	66.0	232	232	323			
499.9	0	875	102.0	1,098	1,330	830			
500.0	0	2,895	237.0	179	1,508	4,472			
502.0	0	4,310	276.0	7,158	8,667	6,145			
503.0	0	5,165	294.0	4,731	13,398	7,009			
Device	Routing	Ir	vert Outl	et Devices					
#1	Primary	498	3.50' 15.0	" Round Culvert	L= 37.0' CPP, s	quare edge headwa	all, Ke= 0.500		
Inle		Inlet	nlet / Outlet Invert= 498.50' / 498.00' S= 0.0135 '/' Cc= 0.900						
n= 0.013, Flow Area= 1.23 sf									
#2 Device 1 500.00' 1.0" Vert. Orifice/Grate C= 0.600 Limited t			nited to weir flow at	low heads					
#3	Device '	1 502	2.00' 24.0)" x 24.0" Horiz. O	rifice/Grate $C=0$	0.600			
			Limi	ited to weir flow at	low heads				

Primary OutFlow Max=0.02 cfs @ 24.17 hrs HW=500.82' TW=498.10' (Dynamic Tailwater)

-1=Culvert (Passes 0.02 cfs of 7.70 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.02 cfs @ 4.26 fps)

□3=Orifice/Grate (Controls 0.00 cfs)

Summary for Subcatchment 1S: REMAINDER FLOW TO CB3P

Runoff 0.03 cfs @ 23.10 hrs, Volume= 0.013 af, Depth= 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=4.98"

Prepared by HP

Type III 24-hr 25YR Rainfall=4.98" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 24

	Area (sf)	CN I	Description							
*	2,004	98 I	8 Existing roadway, HSG A							
	35,850		· · · · · · · · · · · · · · · · · · ·							
	64,959	30 \	Noods, Go	od, HSG A						
	1,650	98 I	Paved park	ing, HSG A	L					
	1,271	98 I	Roofs, HSG	àΑ						
	4,450	39	>75% Grass	s cover, Go	ood, HSG A					
	459,896	30 \	Noods, Go	od, HSG A						
	570,080	31 \	Neighted A	verage						
	565,155	Ç	99.14% Per	vious Area						
	4,925	().86% Impe	ervious Area	a					
To	c Length	Slope	Velocity	Capacity	Description					
<u>(min</u>) (feet)	(ft/ft)	(ft/sec)	(cfs)						
30.5	5 100	0.0100	0.05		Sheet Flow, WOODS					
					Woods: Light underbrush n= 0.400 P2= 2.76"					
14.3	3 900	0.0440	1.05		Shallow Concentrated Flow, WOODS					
					Woodland Kv= 5.0 fps					
4.6	950	0.0500	3.45	38.85	Trap/Vee/Rect Channel Flow,					
					Bot.W=15.00' D=0.50' Z= 15.0 '/' Top.W=30.00'					
					n= 0.050					
49.4	4 1,950	Total								

Summary for Pond 2P: CATCH BASIN

Inflow Area =	16.416 ac,	4.37% Impervious, Inflow I	Depth > 0.04"	for 25YR event
Inflow =	0.05 cfs @	23.36 hrs, Volume=	0.056 af	
Outflow =	0.05 cfs @	23.51 hrs, Volume=	0.056 af, Att	en= 0%, Lag= 9.1 min
Discarded =	0.03 cfs @	23.51 hrs, Volume=	0.051 af	
Primary =	0.02 cfs @	23.51 hrs, Volume=	0.005 af	

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 484.36' @ 23.51 hrs Surf.Area= 0.012 ac Storage= 0.002 af Flood Elev= 488.00' Surf.Area= 0.010 ac Storage= 0.019 af

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 10.5 min (1,430.7 - 1,420.2)

Volume	Invert	Avail.Storage	Storage Description
#1	484.00'	0.017 af	6.00'W x 75.00'L x 4.00'H Prismatoid
#2	484.25'	0.002 af	0.041 af Overall x 40.0% Voids 15.0" Round Pipe Storage L= 75.0'
-		0.019 af	Total Available Storage
		0.010 41	Total / Wallable Glorage
Device	Routing	Invert Ou	utlet Devices
#1	Primary		.0" Round Culvert L= 50.0' CPP, square edge headwall, Ke= 0.500 et / Outlet Invert= 484.30' / 483.80' S= 0.0100 '/' Cc= 0.900
#2	Discarded	n=	0.010 PVC, smooth interior, Flow Area= 1.23 sf 000 in/hr Exfiltration over Surface area

Prepared by HP

Type III 24-hr 25YR Rainfall=4.98" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 25

Discarded OutFlow Max=0.03 cfs @ 23.51 hrs HW=484.36' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.02 cfs @ 23.51 hrs HW=484.36' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.02 cfs @ 0.81 fps)

Summary for Subcatchment 2S: FLOW TO WETLAND

Runoff = 0.04 cfs @ 21.69 hrs, Volume= 0.025 af, Depth= 0.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=4.98"

A	rea (sf)	CN [Description						
	2,004	98 l	98 Unconnected roofs, HSG A						
	1,076	98 F	·						
	20,000	39 >	>75% Gras	s cover, Go	ood, HSG A				
	45,170	30 \	Woods, Go	od, HSG A					
	85,506				ood, HSG A				
1	61,412	30 \	Noods, Go	od, HSG A					
	370			ing, HSG A	l .				
2	28,026	30 \	Noods, Go	od, HSG A					
5	43,564		Neighted A						
5	40,114			vious Area					
	3,450			ervious Area	a				
	2,004	5	58.09% Und	connected					
Tc	Length	Slope	•		Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
13.3	100	0.0800	0.13		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 2.76"				
9.5	900	0.1000	1.58		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
12.4	365	0.0010	0.49	10.99	Trap/Vee/Rect Channel Flow,				
					Bot.W=30.00' D=0.50' Z= 30.0 '/' Top.W=60.00'				
					n= 0.050				
35.2	1,365	Total							

Summary for Subcatchment 3S: FLOW TO POND

Runoff = 0.22 cfs @ 12.55 hrs, Volume= 0.064 af, Depth= 0.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=4.98"

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 26

	Α	rea (sf)	CN	Description		
		2,004	98	Roofs, HSC	àΑ	
*		6,098	98	Paved road	way, HSG	A
		3,615			ing, HSG A	
		53,013				ood, HSG A
		43,821			od, HSG A	•
_	1	08,551	42	Weighted A	verage	
		96,834			vious Area	
		11,717		10.79% lmp	pervious Are	ea
		-				
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	<u> </u>
	14.0	100	0.0700	0.12		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.76"
	0.7	90	0.2000	2.24		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.1	25	0.3300	4.02		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	2.2	412	0.0300	3.13	9.39	Trap/Vee/Rect Channel Flow,
						Bot.W=0.00' D=1.00' Z= 3.0 '/' Top.W=6.00'
_						n= 0.050
	17.0	627	Total			

Summary for Pond 4P: MICRO POOL POND 2

Inflow Area = 3.161 ac, 18.40% Impervious, Inflow Depth > 0.32" for 25YR event

Inflow = 0.61 cfs @ 12.10 hrs, Volume= 0.085 af

Outflow = 0.02 cfs @ 27.54 hrs, Volume= 0.037 af, Atten= 96%, Lag= 926.3 min

Primary = 0.02 cfs @ 27.54 hrs, Volume= 0.037 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 487.75' @ 27.54 hrs Surf.Area= 1,488 sf Storage= 2,128 cf

Plug-Flow detention time= 769.9 min calculated for 0.037 af (43% of inflow)

Center-of-Mass det. time= 392.0 min (1,543.2 - 1,151.2)

Volume	Invert	Avail.Storage	Storage	e Description	
#1	484.00'	4,477 cf	Custon	n Stage Data (Pri	smatic) Listed below (Recalc)
Elevation	Surf.A		c.Store	Cum.Store	
(feet)	(SC	q-ft) (cub	ic-feet)	(cubic-feet)	
484.00		80	0	0	
486.00		460	540	540	
486.90	•	750	544	1,084	
487.00	1,	051	90	1,175	
488.00	1,	633	1,342	2,517	
489.00	2,	288	1,961	4,477	

Type III 24-hr 25YR Rainfall=4.98" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 27

Device	Routing	Invert	Outlet Devices
#1	Primary	484.80'	15.0" Round Culvert L= 46.7' Ke= 0.500
	-		Inlet / Outlet Invert= 484.80' / 484.40' S= 0.0086 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Device 1	487.00'	1.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	488.25'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=0.02 cfs @ 27.54 hrs HW=487.75' TW=484.00' (Dynamic Tailwater)

1=Culvert (Passes 0.02 cfs of 8.87 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.02 cfs @ 4.06 fps)

3=Orifice/Grate (Controls 0.00 cfs)

Prepared by HP

Summary for Subcatchment 4S: FLOW TO CB 2P

Runoff = 0.03 cfs @ 12.31 hrs, Volume= 0.006 af, Depth= 0.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=4.98"

	Area (sf)	CN	Description						
*	972	98	Paved road	way, HSG	Α				
	4,670	39	>75% Gras	s cover, Go	ood, HSG A				
	1,650	30	Woods, Go	od, HSG A					
	7,292	45	Weighted A	verage					
	6,320		86.67% Pervious Area						
	972		13.33% Impervious Area						
_									
Tc	- 3-	Slop	,	Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0	Direct Entry,								

Summary for Reach 5R: ROADSIDE DITCH

Inflow Area = 2.823 ac. 15.41% Impervious, Inflow Depth > 0.16" for 25YR event

Inflow = 0.02 cfs @ 24.17 hrs, Volume= 0.039 af

Outflow = 0.02 cfs @ 24.22 hrs, Volume= 0.038 af, Atten= 0%, Lag= 2.7 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.83 fps, Min. Travel Time= 3.6 min Avg. Velocity = 0.79 fps, Avg. Travel Time= 3.8 min

Peak Storage= 5 cf @ 24.22 hrs

Average Depth at Peak Storage= 0.10', Surface Width= 0.58' Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 11.71 cfs

0.00' x 1.00' deep channel, n= 0.050 Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 180.0' Slope= 0.0467 '/'

Inlet Invert= 498.00', Outlet Invert= 489.60'

Prepared by HP

#1

Primary

Type III 24-hr 25YR Rainfall=4.98" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 28



Summary for Subcatchment 5S: TO LOWER POND

Runoff = 0.61 cfs @ 12.10 hrs, Volume= 0.046 af, Depth= 1.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=4.98"

_	Aı	rea (sf)	CN	Description						
-		8,362	39	>75% Gras	s cover, Go	ood, HSG A				
_		6,389	98	Paved park	ing, HSG A	<u>.</u>				
-		14,751	65	Weighted A	Weighted Average					
		8,362		56.69% Per	rvious Area					
		6,389		43.31% lmp	pervious Ar	ea				
	Tc	Length	Slop	e Velocity	Capacity	Description				
	(min)	(feet)	(ft/f	,	(cfs)	Description				
•	6.0	(1001)	(., (1.000)	(0.0)	Direct Entry				

Summary for Pond 6P: EXISTING EXTENDED CULVERT

Inflow Area = 13.087 ac, 0.86% Impervious, Inflow Depth = 0.01" for 25YR event Inflow = 0.03 cfs @ 23.10 hrs, Volume= 0.013 af

Outflow = $0.03 \text{ cfs} \otimes 23.24 \text{ hrs}$, Volume= 0.013 af, Atten= 0%, Lag= 8.3 min

Primary = 0.03 cfs @ 23.24 hrs, Volume= 0.013 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 487.90' @ 23.24 hrs Surf.Area= 36 sf Storage= 4 cf

Plug-Flow detention time= 3.7 min calculated for 0.013 af (100% of inflow) Center-of-Mass det. time= 2.5 min (1,295.7 - 1,293.2)

Volume	Inve	ert Avai	I.Storage	Storage D	Description	
#1	487.7	73'	1,003 cf	Custom S	Stage Data (Pri	ismatic) Listed below (Recalc)
Elevation (feet		Surf.Area (sq-ft)		c.Store c-feet)	Cum.Store (cubic-feet)	
487.7	3	12	•	0	0	
488.0	0	50		8	8	
490.0	0	945		995	1,003	
Device	Routing	In	vert Out	let Devices		

487.83' **15.0" Round Culvert** L=62.0' Ke= 0.500

Inlet / Outlet Invert= 487.83' / 484.40' S= 0.0553' / Cc= 0.900 n= 0.013, Flow Area= 1.23 sf

Prepared by HP

Type III 24-hr 25YR Rainfall=4.98" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 29

Primary OutFlow Max=0.03 cfs @ 23.24 hrs HW=487.90' TW=484.36' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.03 cfs @ 0.92 fps)

Summary for Subcatchment 7S: TO INLET PIPE

Runoff = 0.72 cfs @ 12.10 hrs, Volume= 0.054 af, Depth= 1.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=4.98"

Ar	rea (sf)	CN	Description						
	7,233	98	Paved park	ing, HSG A	1				
	7,166	39	>75% Gras	s cover, Go	od, HSG A				
	14,399	69	Weighted A	verage					
	7,166		49.77% Per	vious Area					
	7,233		50.23% Imp	pervious Ar	ea				
т.	l	Olara		0	Danawintina				
Tc	Length	Slope	,	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Pond 8P: CULVERT

Inflow Area = 0.331 ac, 50.23% Impervious, Inflow Depth = 1.94" for 25YR event

Inflow = 0.72 cfs @ 12.10 hrs, Volume= 0.054 af

Outflow = 0.72 cfs @ 12.11 hrs, Volume= 0.050 af, Atten= 1%, Lag= 0.7 min

Primary = 0.72 cfs @ 12.11 hrs, Volume= 0.050 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 500.82' @ 24.06 hrs Surf.Area= 232 sf Storage= 209 cf

Plug-Flow detention time= 52.3 min calculated for 0.050 af (93% of inflow)

Center-of-Mass det. time= 18.1 min (868.6 - 850.5)

Volume	Invert	Avail.Sto	rage S	Storage De	escription	
#1	499.40'	2	53 cf (Custom St	age Data (Pri	ismatic) Listed below (Recalc)
Elevation (feet)	Sur	f.Area (sq-ft)	Inc.S (cubic-		Cum.Store (cubic-feet)	
499.40		25		0	0	
500.00		150		53	53	
501.00		250		200	253	
Device R	louting	Invert	Outlet	Devices		
#1 P	rimary	499.40'				.0' Ke= 0.500 499.00' S= 0.0050 '/' Cc= 0.900

n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=0.70 cfs @ 12.11 hrs HW=499.85' TW=498.70' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 0.70 cfs @ 2.59 fps)

Prepared by HP

Type III 24-hr 25YR Rainfall=4.98" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 30

Summary for Link POA1: POA1 AT KEARSARGE MTN RD

Inflow Area = 16.416 ac, 4.37% Impervious, Inflow Depth = 0.00" for 25YR event

Inflow = 0.02 cfs @ 23.51 hrs, Volume= 0.005 af

Primary = 0.02 cfs @ 23.51 hrs, Volume= 0.005 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Summary for Link POA2: AT WETLAND

Inflow Area = 12.479 ac, 0.63% Impervious, Inflow Depth = 0.02" for 25YR event

Inflow = 0.04 cfs @ 21.69 hrs, Volume= 0.025 af

Primary = 0.04 cfs @ 21.69 hrs, Volume= 0.025 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Prepared by HP

Type III 24-hr 50YR Rainfall=5.87" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 31

Time span=1.00-36.00 hrs, dt=0.05 hrs, 701 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Pond 1P: MICRO POOL 1 Peak Elev=501.56' Storage=6,846 cf Inflow=1.04 cfs 0.185 af

Outflow=0.03 cfs 0.057 af

Subcatchment 1S: REMAINDER FLOW TO Runoff Area=570,080 sf 0.86% Impervious Runoff Depth=0.08"

Flow Length=1,950' Tc=49.4 min CN=31 Runoff=0.14 cfs 0.093 af

Pond 2P: CATCH BASIN Peak Elev=484.47' Storage=0.002 af Inflow=0.17 cfs 0.156 af

Discarded=0.04 cfs 0.067 af Primary=0.14 cfs 0.089 af Outflow=0.17 cfs 0.156 af

Subcatchment 2S: FLOW TO WETLAND Runoff Area=543,564 sf 0.63% Impervious Runoff Depth=0.11"

Flow Length=1,365' Tc=35.2 min CN=32 Runoff=0.19 cfs 0.119 af

Subcatchment 3S: FLOW TO POND Runoff Area=108,551 sf 10.79% Impervious Runoff Depth=0.57"

Flow Length=627' Tc=17.0 min CN=42 Runoff=0.61 cfs 0.119 af

Pond 4P: MICRO POOL POND 2 Peak Elev=488.25' Storage=2,949 cf Inflow=0.86 cfs 0.121 af

Outflow=0.03 cfs 0.053 af

Subcatchment 4S: FLOW TO CB 2P Runoff Area=7,292 sf 13.33% Impervious Runoff Depth=0.75"

Tc=6.0 min CN=45 Runoff=0.08 cfs 0.010 af

Reach 5R: ROADSIDE DITCH Avg. Flow Depth=0.11' Max Vel=0.90 fps Inflow=0.03 cfs 0.057 af

n=0.050 L=180.0' S=0.0467 '/' Capacity=11.71 cfs Outflow=0.03 cfs 0.057 af

Subcatchment 5S: TO LOWER POND Runoff Area=14,751 sf 43.31% Impervious Runoff Depth=2.26"

Tc=6.0 min CN=65 Runoff=0.86 cfs 0.064 af

Pond 6P: EXISTING EXTENDED CULVERT Peak Elev=488.00' Storage=8 cf Inflow=0.14 cfs 0.093 af

15.0" Round Culvert n=0.013 L=62.0' S=0.0553 '/' Outflow=0.14 cfs 0.093 af

Subcatchment 7S: TO INLET PIPE Runoff Area=14,399 sf 50.23% Impervious Runoff Depth=2.61"

Tc=6.0 min CN=69 Runoff=0.98 cfs 0.072 af

Pond 8P: CULVERT Peak Elev=501.56' Storage=253 cf Inflow=0.98 cfs 0.072 af

15.0" Round Culvert n=0.013 L=80.0' S=0.0050 '/' Outflow=0.98 cfs 0.066 af

Link POA1: POA1 AT KEARSARGE MTN RD Inflow=0.14 cfs 0.089 af

Primary=0.14 cfs 0.089 af

Link POA2: AT WETLAND Inflow=0.19 cfs 0.119 af

Primary=0.19 cfs 0.119 af

Total Runoff Area = 28.894 ac Runoff Volume = 0.477 af Average Runoff Depth = 0.20" 97.24% Pervious = 28.098 ac 2.76% Impervious = 0.796 ac

Type III 24-hr 50YR Rainfall=5.87" Printed 8/5/2024

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 32

Summary for Pond 1P: MICRO POOL 1

Inflow Area = 2.823 ac, 15.41% Impervious, Inflow Depth = 0.79" for 50YR event

Inflow = 1.04 cfs @ 12.12 hrs, Volume= 0.185 af

Outflow = 0.03 cfs @ 24.17 hrs, Volume= 0.057 af, Atten= 97%, Lag= 723.0 min

Primary = 0.03 cfs @ 24.17 hrs, Volume= 0.057 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 501.56' @ 24.17 hrs Surf.Area= 3,975 sf Storage= 6,846 cf

Plug-Flow detention time= 748.5 min calculated for 0.057 af (31% of inflow)

Center-of-Mass det. time= 566.6 min (1,479.2 - 912.6)

Volume	Inv	ert Ava	il.Storage	Storage Descript	ion				
#1	497.0	00'	13,398 cf	Custom Stage Data (Irregular) Listed below (Recalc)					
Elevatio		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
497.0	00	150	48.0	0	0	150			
498.0	00	325	66.0	232	232	323			
499.9	90	875	102.0	1,098	1,330	830			
500.0	00	2,895	237.0	179	1,508	4,472			
502.0	00	4,310	276.0	7,158	8,667	6,145			
503.0	00	5,165	294.0	4,731	13,398	7,009			
Device	Routing	lr	nvert Out	et Devices					
#1	Primary	498	3.50' 15.0	" Round Culvert	L= 37.0' CPP, s	quare edge headw	all, Ke= 0.500		
			Inle	t / Outlet Invert= 49	98.50' / 498.00' S	= 0.0135 '/' Cc= 0	.900		
n= (0.013, Flow Area=	1.23 sf						
#2 Device 1 500.00' 1.0"		"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads							
#3	Device ²	1 502)" x 24.0" Horiz. O ited to weir flow at		0.600			

Primary OutFlow Max=0.03 cfs @ 24.17 hrs HW=501.56' TW=498.11' (Dynamic Tailwater)

1=Culvert (Passes 0.03 cfs of 9.22 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.03 cfs @ 5.93 fps)

3=Orifice/Grate (Controls 0.00 cfs)

Summary for Subcatchment 1S: REMAINDER FLOW TO CB3P

Runoff = 0.14 cfs @ 15.98 hrs, Volume= 0.093 af, Depth= 0.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 50YR Rainfall=5.87"

Type III 24-hr 50YR Rainfall=5.87" Printed 8/5/2024

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 33

	Α	rea (sf)	CN I	Description		
*		2,004	98	Existing roa	dway, HSG	S A
		35,850	39	>75% Gras	s cover, Go	ood, HSG A
		64,959	30	Woods, Go	od, HSG A	
		1,650	98	Paved park	ing, HSG A	ı.
		1,271	98	Roofs, HSG	àΑ	
		4,450			•	ood, HSG A
_	4	59,896	30	Woods, Go	od, HSG A	
	5	70,080		Weighted A	•	
	5	65,155		99.14% Per		
		4,925	(0.86% lmpe	ervious Area	a
	_		-			
	Tc	Length	Slope	•	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	30.5	100	0.0100	0.05		Sheet Flow, WOODS
						Woods: Light underbrush n= 0.400 P2= 2.76"
	14.3	900	0.0440	1.05		Shallow Concentrated Flow, WOODS
						Woodland Kv= 5.0 fps
	4.6	950	0.0500	3.45	38.85	•
						Bot.W=15.00' D=0.50' Z= 15.0'/' Top.W=30.00'
_						n= 0.050
	49.4	1,950	Total			

Summary for Pond 2P: CATCH BASIN

Inflow Area =	16.416 ac,	4.37% Impervious, Inflow	Depth > 0.11"	for 50YR event
Inflow =	0.17 cfs @	15.98 hrs, Volume=	0.156 af	
Outflow =	0.17 cfs @	16.00 hrs, Volume=	0.156 af, Atte	en= 0%, Lag= 1.7 min
Discarded =	0.04 cfs @	16.00 hrs, Volume=	0.067 af	
Primary =	0.14 cfs @	16.00 hrs, Volume=	0.089 af	

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 484.47' @ 16.00 hrs Surf.Area= 0.012 ac Storage= 0.002 af Flood Elev= 488.00' Surf.Area= 0.010 ac Storage= 0.019 af

Plug-Flow detention time= 13.3 min calculated for 0.156 af (100% of inflow) Center-of-Mass det. time= 11.9 min (1,264.4 - 1,252.5)

Volume	Invert	Avail.Storage	Storage Description
#1	484.00'	0.017 af	6.00'W x 75.00'L x 4.00'H Prismatoid
#2	484.25'	0.002 af	0.041 af Overall x 40.0% Voids 15.0" Round Pipe Storage L= 75.0'
		0.019 af	Total Available Storage
Device	Routing	Invert Ou	utlet Devices
#1	Primary	484.30' 15	.0" Round Culvert L= 50.0' CPP, square edge headwall, Ke= 0.500
			et / Outlet Invert= 484.30' / 483.80' S= 0.0100 '/' Cc= 0.900
			0.010 PVC, smooth interior, Flow Area= 1.23 sf
#2	Discarded	484.00' 3.0	000 in/hr Exfiltration over Surface area

Prepared by HP

Type III 24-hr 50YR Rainfall=5.87" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 34

Discarded OutFlow Max=0.04 cfs @ 16.00 hrs HW=484.47' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.14 cfs @ 16.00 hrs HW=484.47' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.14 cfs @ 1.39 fps)

Summary for Subcatchment 2S: FLOW TO WETLAND

Runoff = 0.19 cfs @ 15.44 hrs, Volume= 0.119 af, Depth= 0.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 50YR Rainfall=5.87"

Are	ea (sf)	CN I	Description		
2	2,004	98 l	Jnconnecte	ed roofs, HS	SG A
	1,076	98 I	Paved park	ing, HSG A	L
	0,000				ood, HSG A
	5,170			od, HSG A	
	5,506				ood, HSG A
16 ⁻	1,412		,	od, HSG A	
	370			ing, HSG A	
	8,026			od, HSG A	
	3,564		Weighted A	•	
	0,114			vious Area	
	3,450			ervious Are	a
2	2,004		58.09% Und	connected	
Tc l	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	2000 pilon
13.3	100	0.0800	,	· /	Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 2.76"
9.5	900	0.1000	1.58		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
12.4	365	0.0010	0.49	10.99	Trap/Vee/Rect Channel Flow,
					Bot.W=30.00' D=0.50' Z= 30.0 '/' Top.W=60.00'
					n= 0.050
35.2	1,365	Total			

Summary for Subcatchment 3S: FLOW TO POND

Runoff = 0.61 cfs @ 12.45 hrs, Volume= 0.119 af, Depth= 0.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 50YR Rainfall=5.87"

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 35

	Α	rea (sf)	CN	Description		
		2,004	98	Roofs, HSC	àΑ	
*		6,098	98	Paved road	way, HSG	A
		3,615	98	Paved park	ing, HSG A	·
		53,013	39	>75% Ġras	s cover, Go	ood, HSG A
		43,821		Woods, Go	od, HSG A	
	1	08,551	42	Weighted A	verage	
		96,834		89.21% Per		
		11,717		10.79% lmp	pervious Are	ea
		,		'		
	Tc	Length	Slope	e Velocity	Capacity	Description
	(min)	(feet)	(ft/ft	(ft/sec)	(cfs)	·
	14.0	100	0.0700	0.12		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.76"
	0.7	90	0.2000	2.24		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.1	25	0.3300	4.02		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	2.2	412	0.0300	3.13	9.39	
						Bot.W=0.00' D=1.00' Z= 3.0 '/' Top.W=6.00'
_						n= 0.050
	17.0	627	Total			

Summary for Pond 4P: MICRO POOL POND 2

Inflow Area = 3.161 ac, 18.40% Impervious, Inflow Depth > 0.46" for 50YR event

Inflow = 0.86 cfs @ 12.10 hrs, Volume= 0.121 af

Outflow = 0.03 cfs @ 25.97 hrs, Volume= 0.053 af, Atten= 96%, Lag= 832.3 min

Primary = 0.03 cfs @ 25.97 hrs, Volume= 0.053 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 488.25' @ 25.97 hrs Surf.Area= 1,798 sf Storage= 2,949 cf

Plug-Flow detention time= 740.1 min calculated for 0.053 af (44% of inflow)

Center-of-Mass det. time= 359.7 min (1,508.2 - 1,148.5)

Volume	Invert Ava	ail.Storage	Storage	e Description	
#1	484.00'	4,477 cf	Custon	n Stage Data (Pri	smatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)		Store -feet)	Cum.Store (cubic-feet)	
484.00	80		0	0	
486.00	460		540	540	
486.90	750		544	1,084	
487.00	1,051		90	1,175	
488.00	1,633		1,342	2,517	
489.00	2,288		1,961	4,477	

Prepared by HP

Type III 24-hr 50YR Rainfall=5.87" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 36

Device	Routing	Invert	Outlet Devices
#1	Primary	484.80'	15.0" Round Culvert L= 46.7' Ke= 0.500
	•		Inlet / Outlet Invert= 484.80' / 484.40' S= 0.0086 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Device 1	487.00'	1.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	488.25'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=0.03 cfs @ 25.97 hrs HW=488.25' TW=484.31' (Dynamic Tailwater)

1=Culvert (Passes 0.03 cfs of 9.87 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.03 cfs @ 5.30 fps)

-3=Orifice/Grate (Weir Controls 0.00 cfs @ 0.16 fps)

Summary for Subcatchment 4S: FLOW TO CB 2P

Runoff = 0.08 cfs @ 12.14 hrs, Volume= 0.010 af, Depth= 0.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 50YR Rainfall=5.87"

A	rea (sf)	CN	Description						
*	972	98	Paved roadway, HSG A						
	4,670	39	>75% Gras	s cover, Go	od, HSG A				
	1,650	30	Woods, Go	od, HSG A					
	7,292	45	45 Weighted Average						
	6,320		86.67% Pervious Area						
	972		13.33% Impervious Area						
Tc (min)	Length (feet)	Slop (ft/f	•	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Reach 5R: ROADSIDE DITCH

Inflow Area = 2.823 ac. 15.41% Impervious, Inflow Depth > 0.24" for 50YR event

Inflow = 0.03 cfs @ 24.17 hrs, Volume= 0.057 af

Outflow = 0.03 cfs @ 24.22 hrs, Volume= 0.057 af, Atten= 0%, Lag= 2.7 min

Routing by Dyn-Stor-Ind method. Time Span= 1.00-36.00 hrs. dt= 0.05 hrs

Max. Velocity= 0.90 fps, Min. Travel Time= 3.4 min Avg. Velocity = 0.87 fps, Avg. Travel Time= 3.5 min

Peak Storage= 7 cf @ 24.22 hrs

Average Depth at Peak Storage= 0.11', Surface Width= 0.66' Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 11.71 cfs

 $0.00' \times 1.00'$ deep channel, n= 0.050 Side Slope Z-value= 3.0 '/' Top Width= 6.00'

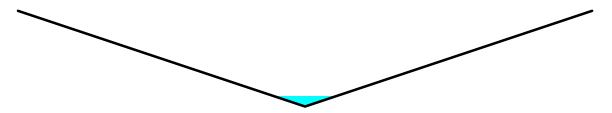
Length= 180.0' Slope= 0.0467 '/'

Inlet Invert= 498.00', Outlet Invert= 489.60'

Type III 24-hr 50YR Rainfall=5.87" Printed 8/5/2024

Prepared by HP HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 37



Summary for Subcatchment 5S: TO LOWER POND

Runoff 0.86 cfs @ 12.10 hrs, Volume= 0.064 af, Depth= 2.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 50YR Rainfall=5.87"

Area (sf)	CN	Description					
8,362	39	>75% Grass cover, Good, HSG A					
6,389	98	Paved parking, HSG A					
14,751	65	Weighted A	verage				
8,362	8,362 56.69% Pervious Area						
6,389		43.31% lmp	pervious Ar	rea			
Tc Length		,	Capacity	·			
(min) (feet)	(ft/	ft) (ft/sec)	(cfs)				
6.0				Direct Entry,			

Summary for Pond 6P: EXISTING EXTENDED CULVERT

Inflow Area = 13.087 ac, 0.86% Impervious, Inflow Depth = 0.08" for 50YR event

Inflow 0.14 cfs @ 15.98 hrs, Volume= 0.093 af

0.14 cfs @ 16.00 hrs, Volume= Outflow 0.093 af, Atten= 0%, Lag= 1.0 min

0.14 cfs @ 16.00 hrs, Volume= Primary 0.093 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Peak Elev= 488.00' @ 16.00 hrs Surf.Area= 50 sf Storage= 8 cf

Plug-Flow detention time= 1.6 min calculated for 0.093 af (100% of inflow)

Center-of-Mass det. time= 1.0 min (1,143.6 - 1,142.6)

Volume	Inve	ert Ava	il.Storage	Storage	Description	
#1	487.7	73'	1,003 cf	Custom	Stage Data (Prisn	natic) Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)	_	.Store c-feet)	Cum.Store (cubic-feet)	
487.7	' 3	12		0	0	
488.0	00	50		8	8	
490.0	00	945		995	1,003	
Device	Routing	In	vert Outl	et Device	S	
#1	Primary	487	7.83' 15.0	" Round	Culvert L= 62.0'	Ke= 0.500

Inlet / Outlet Invert= 487.83' / 484.40' S= 0.0553 '/' Cc= 0.900

n= 0.013, Flow Area= 1.23 sf

Prepared by HP

Type III 24-hr 50YR Rainfall=5.87" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 38

Primary OutFlow Max=0.14 cfs @ 16.00 hrs HW=488.00' TW=484.47' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.14 cfs @ 1.40 fps)

Summary for Subcatchment 7S: TO INLET PIPE

Runoff 0.98 cfs @ 12.10 hrs, Volume= 0.072 af, Depth= 2.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Type III 24-hr 50YR Rainfall=5.87"

Ar	rea (sf)	CN	Description					
	7,233	98	Paved parking, HSG A					
	7,166	39	>75% Ġras	s cover, Go	od, HSG A			
	14,399	69	Weighted A	verage				
	7,166	49.77% Pervious Area						
	7,233		50.23% lmp	pervious Are	ea			
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Pond 8P: CULVERT

Inflow Area = 0.331 ac, 50.23% Impervious, Inflow Depth = 2.61" for 50YR event

0.072 af Inflow 0.98 cfs @ 12.10 hrs, Volume=

0.98 cfs @ 12.11 hrs, Volume= 0.066 af, Atten= 1%, Lag= 0.7 min Outflow

Primary 0.98 cfs @ 12.11 hrs, Volume= 0.066 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs Peak Elev= 501.56' @ 24.10 hrs Surf.Area= 250 sf Storage= 253 cf

Plug-Flow detention time= 45.3 min calculated for 0.066 af (92% of inflow)

Center-of-Mass det. time= 4.5 min (846.2 - 841.7)

Volume	Inve	ert Avail.	Storage	Storage	Description	
#1	499.4	10'	253 cf	Custom	Stage Data (Prisi	matic) Listed below (Recalc)
Elevatio (feet		Surf.Area (sq-ft)		c.Store c-feet)	Cum.Store (cubic-feet)	
499.4	0	25		0	0	
500.0	0	150		53	53	
501.0	0	250		200	253	
Device	Routing	Inv	ert Out	let Device	es	
#1	Primary	499.	_		Culvert L= 80.0	' Ke= 0.500 99.00' S= 0.0050 '/' Cc= 0.900
			11110	i / Oullet i	110611- 433.40 / 4	33.00 3-0.0030 / 00-0.300

n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=0.96 cfs @ 12.11 hrs HW=499.94' TW=499.17' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.96 cfs @ 2.81 fps)

Prepared by HP

Type III 24-hr 50YR Rainfall=5.87" Printed 8/5/2024

HydroCAD® 10.10-4a s/n 11004 © 2020 HydroCAD Software Solutions LLC

Page 39

Summary for Link POA1: POA1 AT KEARSARGE MTN RD

Inflow Area = 16.416 ac, 4.37% Impervious, Inflow Depth = 0.06" for 50YR event

Inflow = 0.14 cfs @ 16.00 hrs, Volume= 0.089 af

Primary = 0.14 cfs @ 16.00 hrs, Volume= 0.089 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

Summary for Link POA2: AT WETLAND

Inflow Area = 12.479 ac, 0.63% Impervious, Inflow Depth = 0.11" for 50YR event

Inflow = 0.19 cfs @ 15.44 hrs, Volume= 0.119 af

Primary = 0.19 cfs @ 15.44 hrs, Volume= 0.119 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-36.00 hrs, dt= 0.05 hrs

STORMWATER MANAGEMENT REPORT Residential Subdivision Kearsarge MT. Road Warner NH

OPERATION AND MAINTENANCE PLAN

STORM-WATER OPERATION AND MAINTENANCE MANUAL

PROPOSED RESIDENTIAL DEVELOPMENT KEARSARGE MOUNTAIN ROAD WARNER, NH

February 21, 2024

Owner: Sydney Elizabeth Boyer

Kearsarge Mountain Road

Warner NH 03278

The responsibility of the maintenance and management of the storm-water facilities is the "owner of record"

This manual has been prepared in order to assist in the long term functionality of the storm-water system.

The owner is responsible to construct and maintain the storm-water system in accordance with the approved subdivision plan, and for implementing the requirements of this document. The approved plan is considered to be part of this manual.

This site utilizes forebays, stone lined and grass lined swales, deep sump catch basins and two micro pool extended detention ponds to mitigate the storm-water associated with the building and roadway design.

Owner: Sydney Elizabeth Boyer

Kearsarge Mountain Road

Warner NH 03278

Responsibility: The storm water management facilities proposed to be constructed on the site located off of Kearsarge Mountain Road in Warner NH will remain under the ownership as described above. The owner will be responsible for the continued maintenance of the drainage features particular to this development.

1. Inspection & Maintenance Schedule The storm water management systems on the project consist of a number of different drainage management systems that need to be addressed. The systems on the proposed site consist of one infiltration pond, and two forebays.

Inspections

All of the system's elements must be inspected after a heavy rain storm event. As well as:

- Micro pool extended detention ponds should be inspected after major storms and every 6 months for accumulated debris and siltation within the basin as well as debris accumulation on the over flow weir. Woody and herbaceous vegetation should be removed from the rock rip rap with the basin annually. Embankments and slopes will be mowed and woody and herbaceous vegetation shall be removed. Outlet control devices and grates to be inspected for debris and clogging.
- **swales** will
- within the basin or swale as well as debris accumulation on the over flow weir. Woody and herbaceous vegetation shall be removed from swale annually. Embankments and slopes will be mowed and woody and herbaceous vegetation shall be removed. be inspected after major storms and every 6 months for accumulated debris and siltation

- **Forebays** should be inspected after major storms and every 6 months for accumulated sediment and debris. Grass and woody vegetation should be removed from the forebay annually. Staff gage or other measuring device shall be installed to indicate the depth of the sediment.
- Catch Basins, Drainage Manholes, Pipes and Outlets Remove accumulated sediment from the structures and the outlets every year during late winter or early spring. Accumulated sediment shall be disposed of off-site in accordance with applicable local, State and/or Federal guidelines

Maintenance

Sediment and Debris found in any of the storm water management system elements shall be immediately removed and disposed of in a manner consistent with all state and local permits. Wherever damage to slopes, lawns or basins is discovered, such damage shall be repaired immediately, in addition a regular schedule of maintenance shall be followed:

Good Housekeeping

Sanding of drives and walks to be performed as needed, excess sanding to be minimized. Salting to be restricted to potassium chloride only.

Drives & Parking Areas – Remove accumulated sand in the spring of each year. Provide for sweeping of drives at least twice every year in addition to the spring cleaning. Accumulated sediment shall be disposed of to prevent accumulation in the storm water management systems.

Lawns and landscaped areas to be well maintained fall leaves and heavy clippings as well as any loose landscape materials to be raked up and removed to prevent clogging of inlets grates, deep sumps and stone.

Sydney Elizabeth Boyer Kearsarge Mountain Road Owner:

Warner NH 03278

MAINTENANCE LOG

Any required or completed maintenance is to be recorded and maintained with this manual for perpetuity.

' ' '	
Annually, copies of the co and the Department of Pul	ompleted maintenance logs are to be provided to the Town of Loudon blic Works.
Micro pool extended Detention ponds Maintenance Required:	Mow embankments at least annually to control woody vegetation Remove debris inspect overflow and inlet.
	Work performed by:
Maintenance Competed:	
	Date Completed:
Swales:	
Maintenance Required:	
	Control woody vegetation Remove debris and sediment, mow embankments Remove accumulated sediment.
	Vork performed by:
Forebays:	
Maintenance Required:	
	Control woody vegetation Remove debris and sediment, Install gage / measuring device to indicate sediment depth Remove accumulated sediment.
Maintenance Competed: V	Vork performed by:

Catch Basins, Drainage Manholes, Pipes and Outlets:

Date Completed:

Required:	
	 Remove accumulated sediment from the structures and the outlets every year during late winter or early spring. Accumulated sediment shall be disposed of off-site in accordance with applicable local, State and/or Federal guidelines
Maintenance Competed: Wo	ork performed by:
Good Housekeeping Pract	ices:
Maintenance Required:	Roadways – Sweeping of drives. Fall and Spring clean up

Maintenance Competed: Work performed by:

DRAINAGE AREA PLANS